

A New VISITOR'S PRESSBOX for:

CARTERSVILLE HIGH SCHOOL

320 E. CHURCH STREET - CARTERSVILLE, GA 30120

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	<div>BUILDING INFORMATION</div> <div><div>OWNER: CARTERSVILLE CITY SCHOOLS 15 NELSON STREET CARTERSVILLE, GA 30120</div><div>CONSTRUCTION TYPE: IBC - IIB</div><div>NUMBER OF STORIES: ONE</div><div>BUILDING SPRINKLERED: NO</div><div>SQUARE FOOTAGE: 370 S.F.</div><div>OCCUPANT LOAD: 4 PERSONS AT 1 PER 100 S.F.</div></div>	<div>CODE INFORMATION</div> <div><div>ALL WORK IN NEW AND RENOVATED AREAS SHALL BE IN COMPLIANCE WITH THE FOLLOWING:</div><div>2012 LIFE SAFETY CODE (LSC) - INCLUDING THE GA 120-3-3 RULES &amp; REGULATIONS OF THE STATE FIRE COMMISSIONER</div><div>2012 INTERNATIONAL BUILDING CODE (IBC)- 2014 AMENDMENTS</div><div>2014 NATIONAL ELECTRIC CODE (NEC)</div></div> <div><div>THESE DRAWINGS ARE THE EXCLUSIVE PROPERTY OF KRH ARCHITECTS, INC. AND HAVE BEEN PREPARED AS AN INSTRUMENT OF SERVICE FOR THE CARTERSVILLE CITY SCHOOL SYSTEM. THE USE OR REPRODUCTION IN ANY FORM OF THESE CONTRACT DOCUMENTS WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT IS PROHIBITED.</div><div>COPYRIGHT 2017 ©</div></div>
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K R H A R C H I T E C T S , I N C .

855 ABUTMENT ROAD - SUITE 4 - DALTON, GA 30721 - 706.529.5895

STRUCTURAL - WILLIAM J. PELTIER & ASSOCIATES  
ELECTRICAL - TANKERSLEY-JACKSON & ASSOCIATES

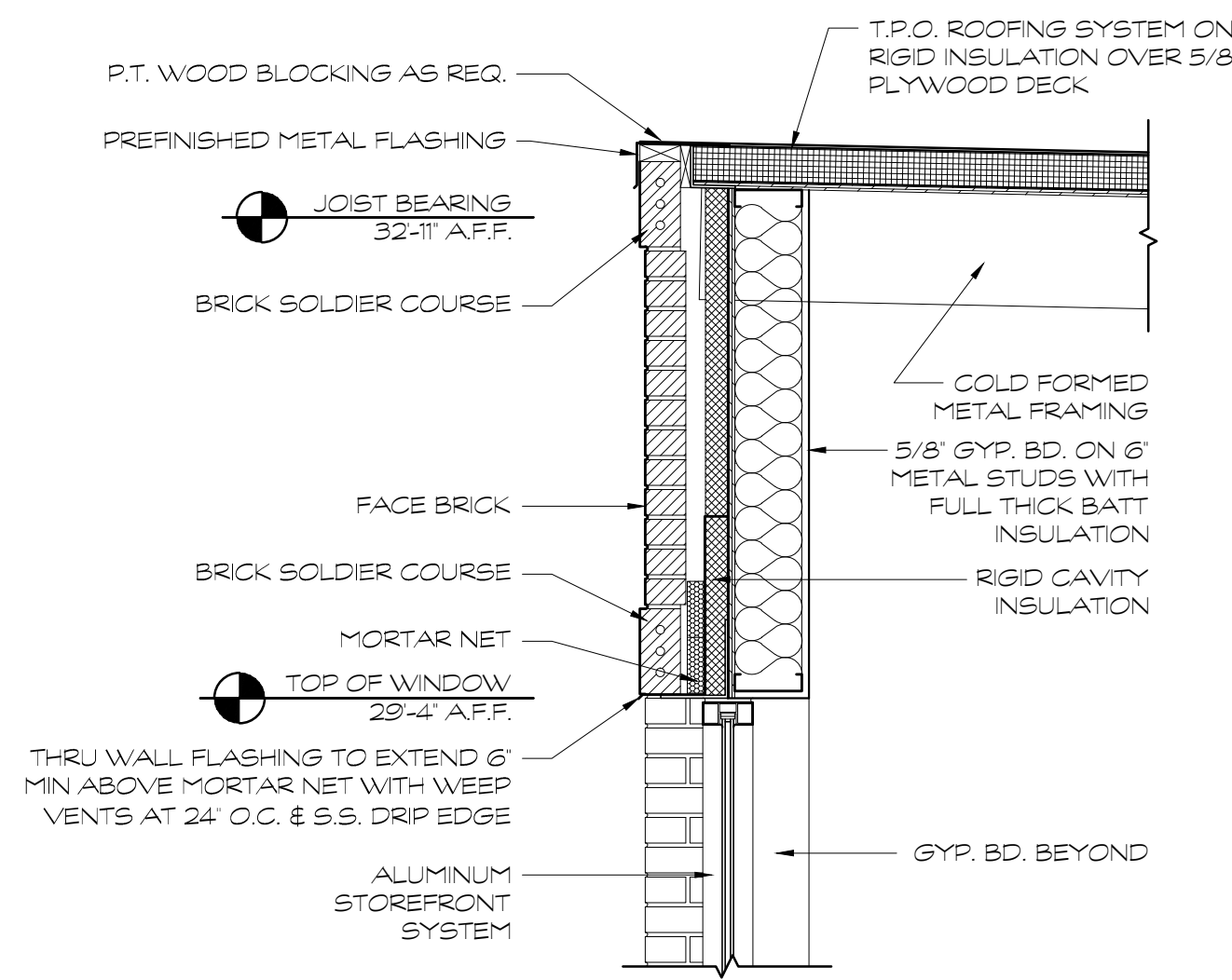
PROJECT NO.	DATE	REVISIONS
17-008	04/07/17	

<div><div><input type="checkbox"/> PRELIMINARY REVIEW</div><div><input type="checkbox"/> CHECKSET REVIEW</div><div><input type="checkbox"/> FINAL REVIEW</div><div><input type="checkbox"/> FOR CONSTRUCTION</div></div>	<div>SET NO.</div> <div>DRAWING NO.: T1.1</div>
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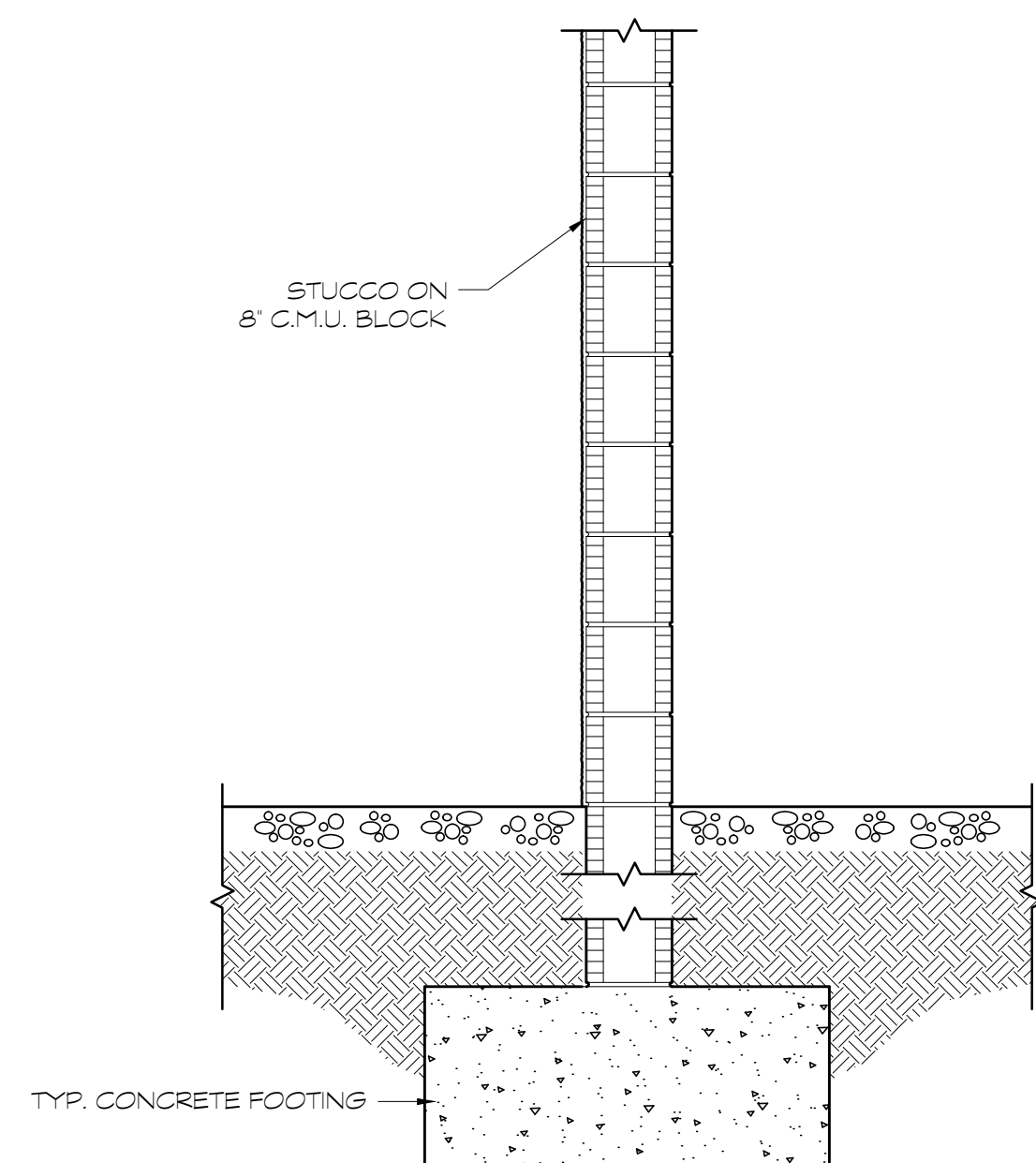




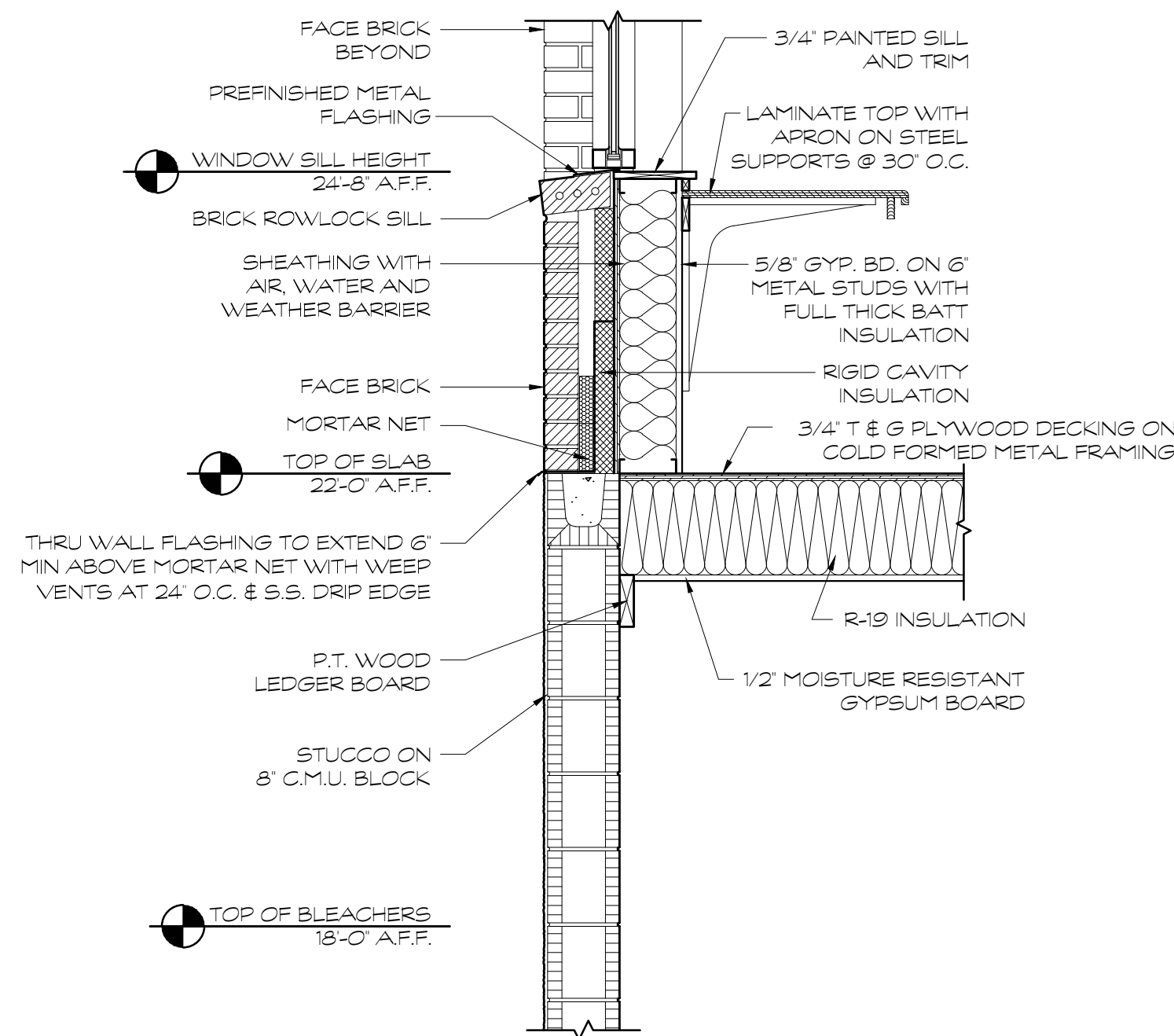




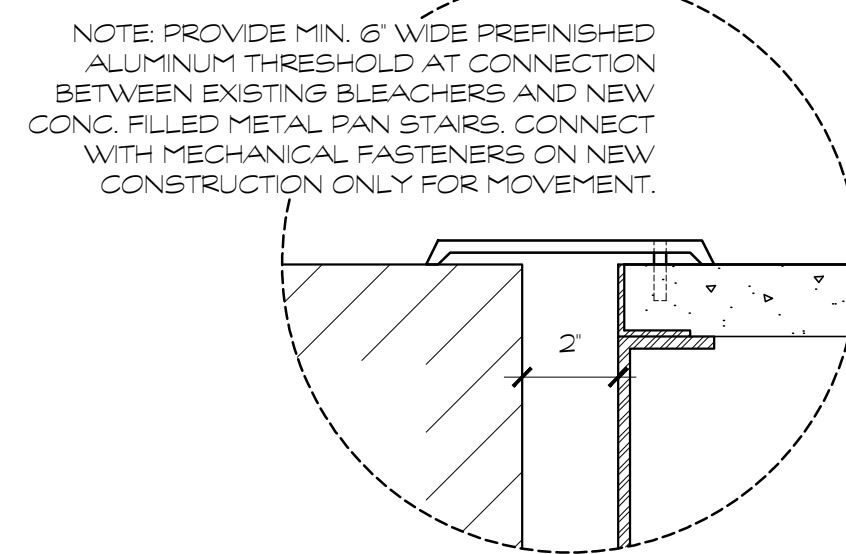
② WALL SECTION  
SCALE: 3/4" = 1'-0"



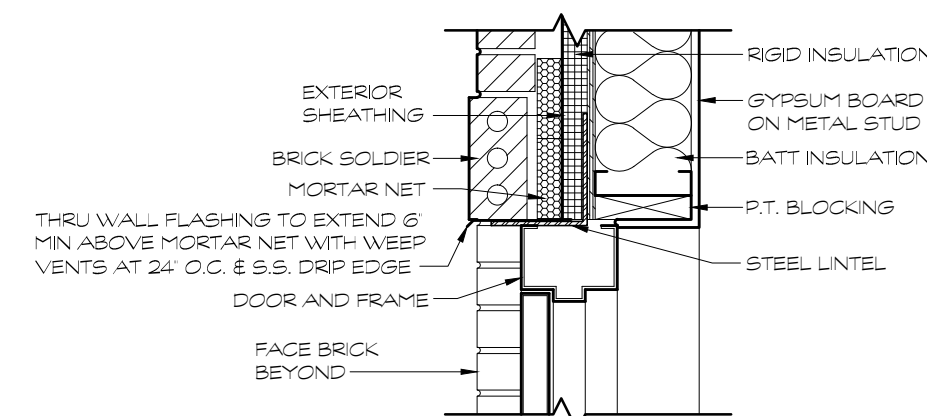
① WALL SECTION  
SCALE: 3/4" = 1'-0"



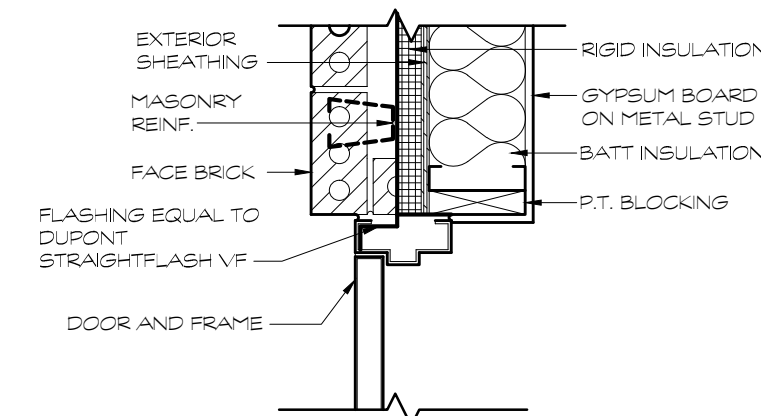
③ DETAIL  
SCALE: 1-1/2" = 1'-0"



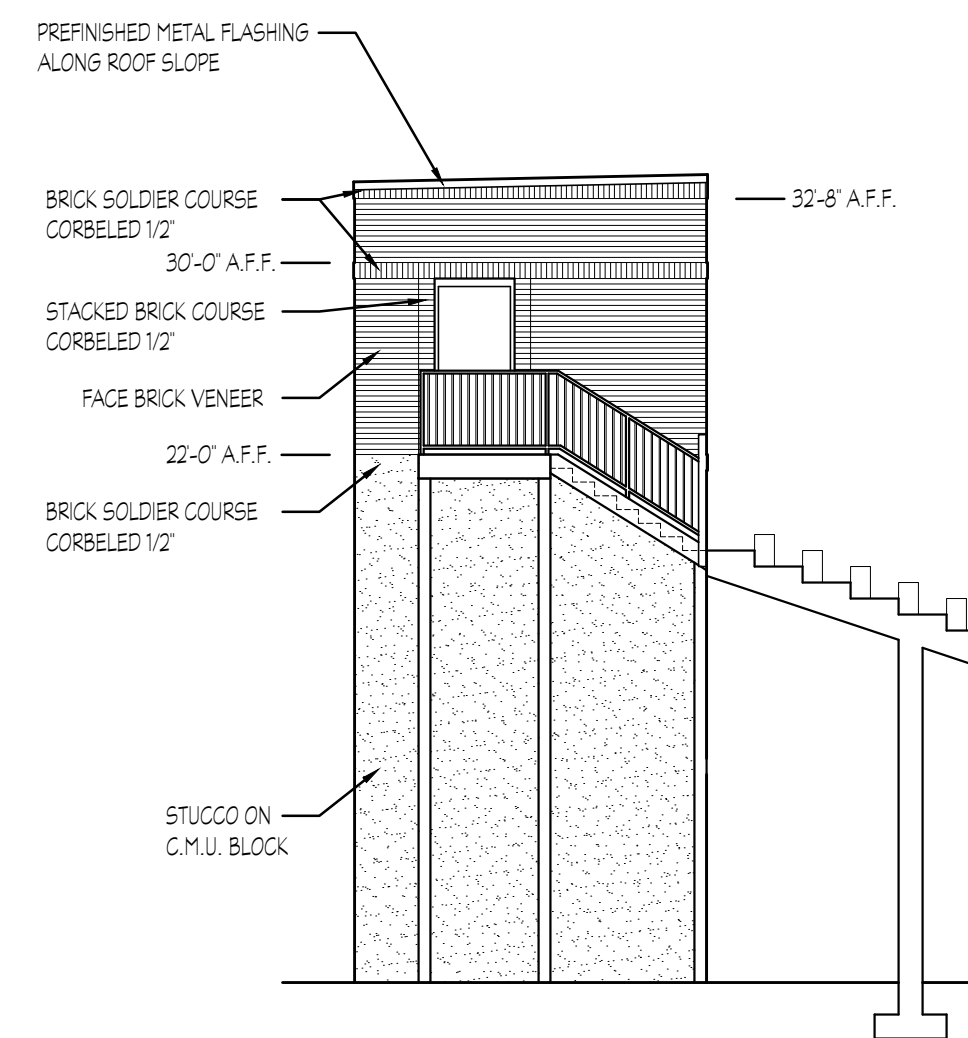
④ DETAIL  
SCALE: 3" = 1'-0"



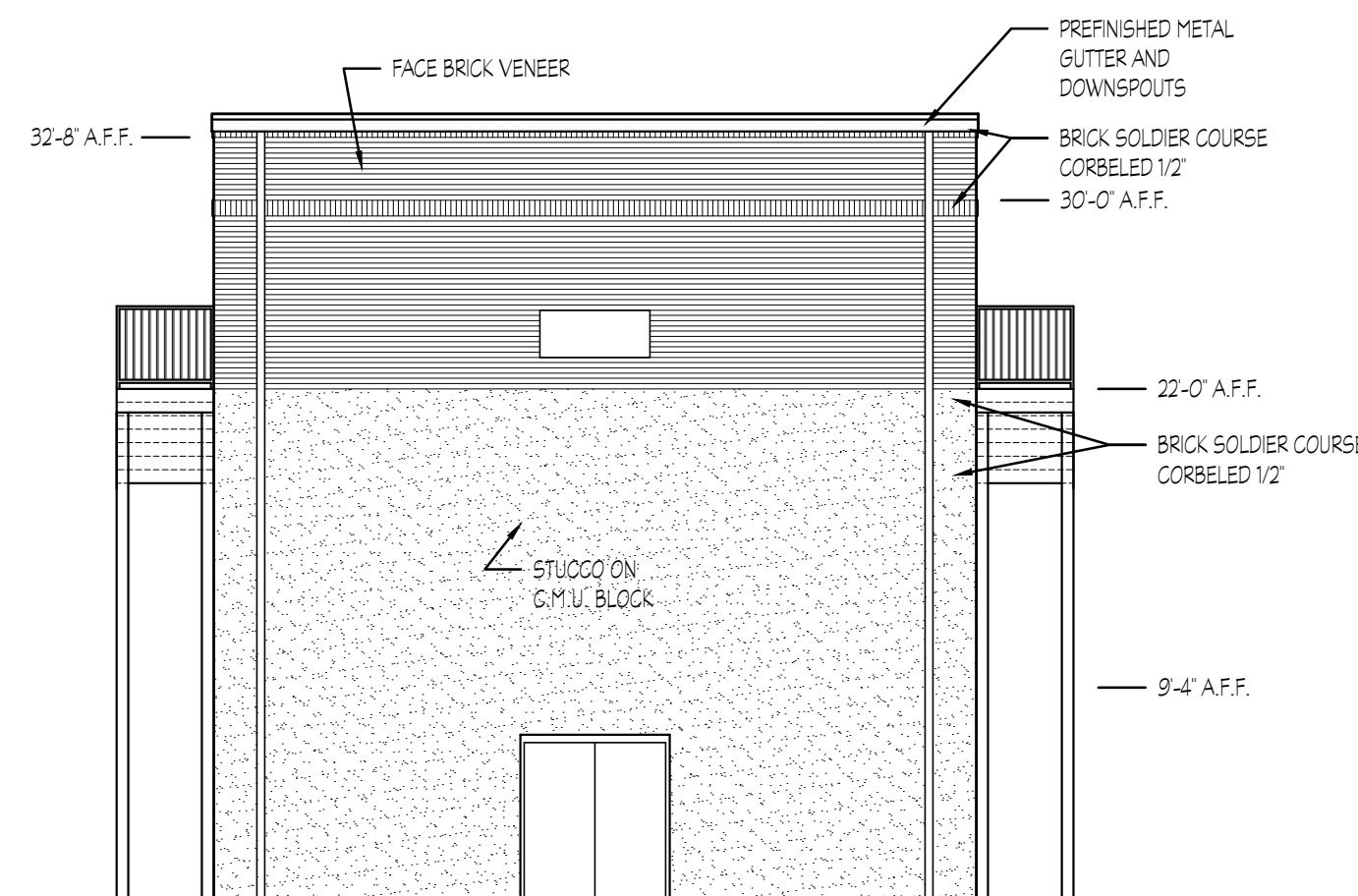
⑤ DOOR HEAD  
SCALE: 1" = 1'-0"



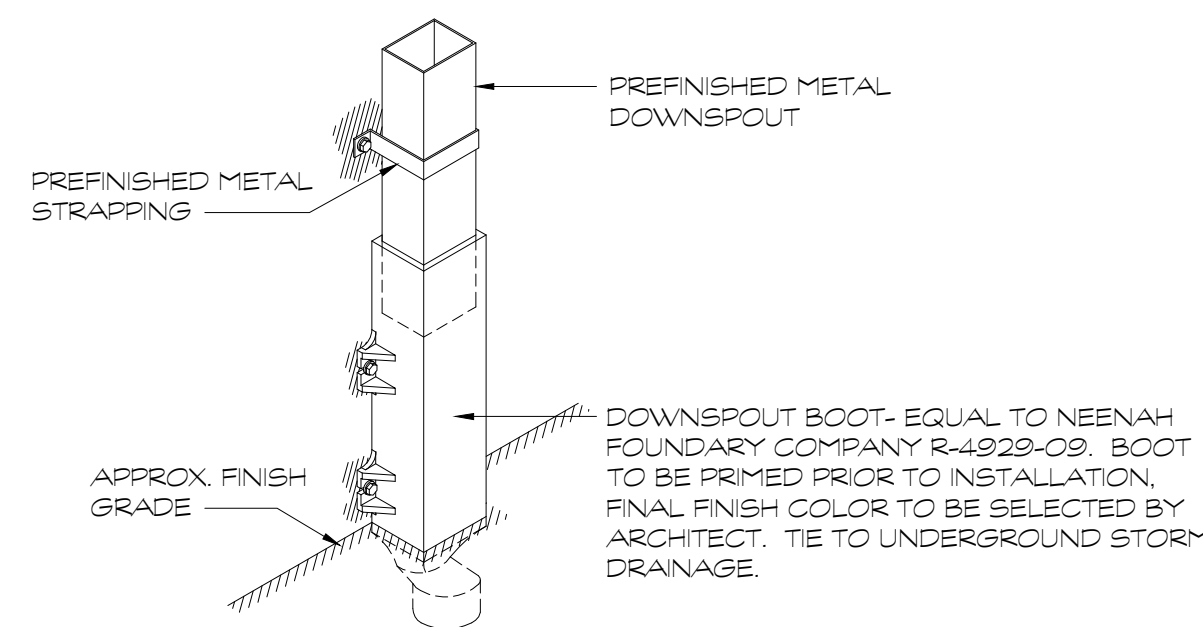
⑥ DOOR JAMB  
SCALE: 1" = 1'-0"  
NOTE: WINDOW JAMB SIMILAR



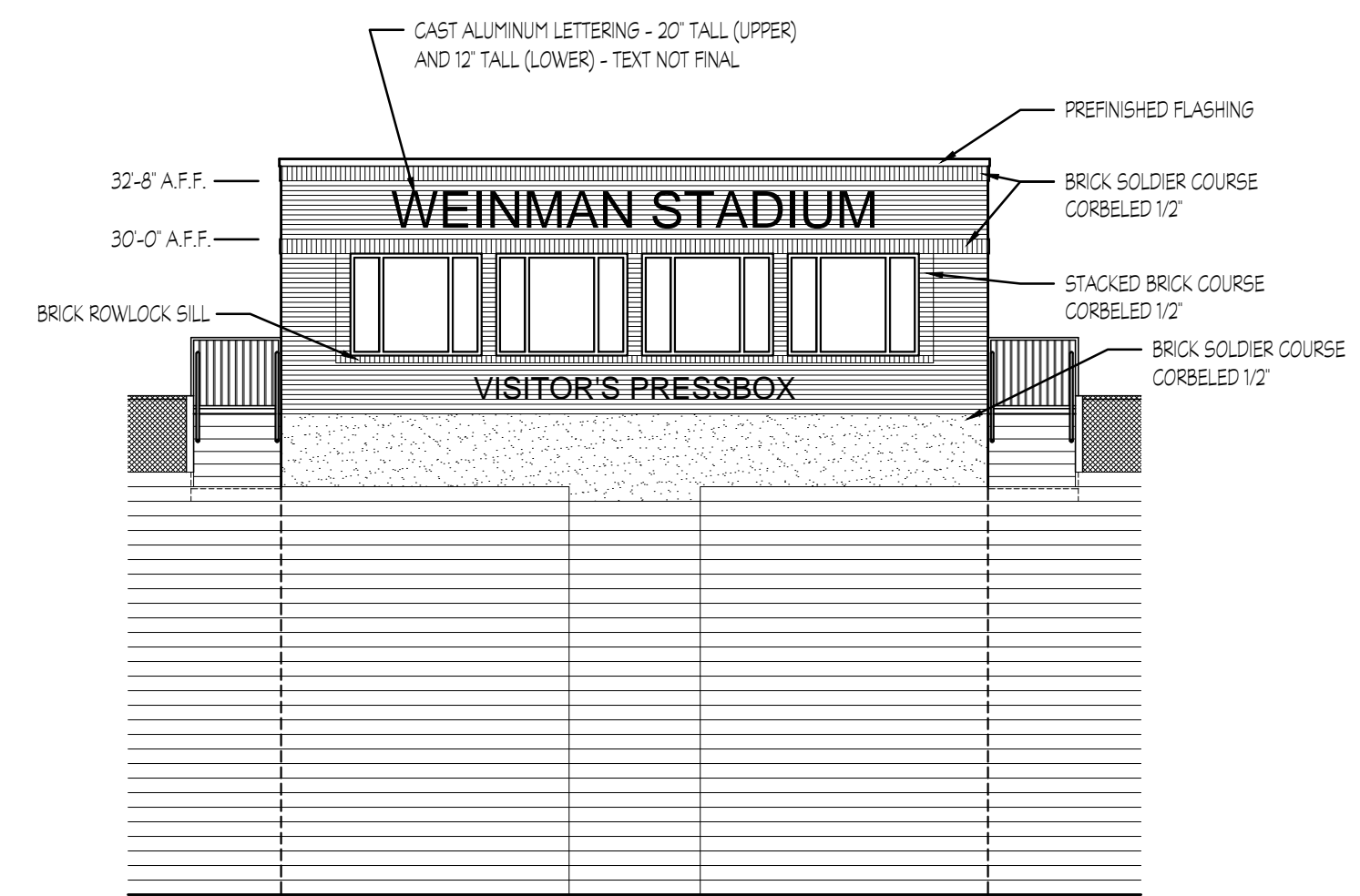
⑦ ELEVATION  
SCALE: 1/8" = 1'-0"



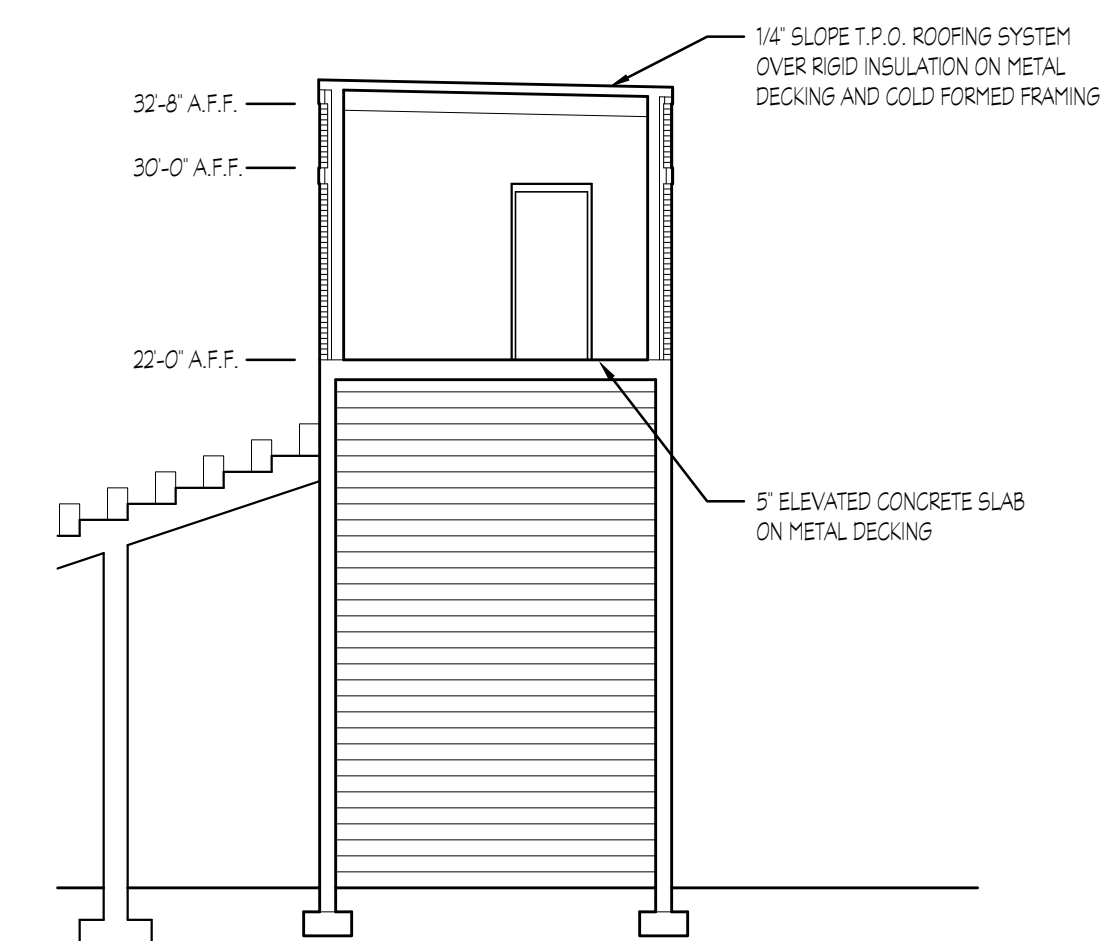
⑨ ELEVATION  
SCALE: 1/8" = 1'-0"



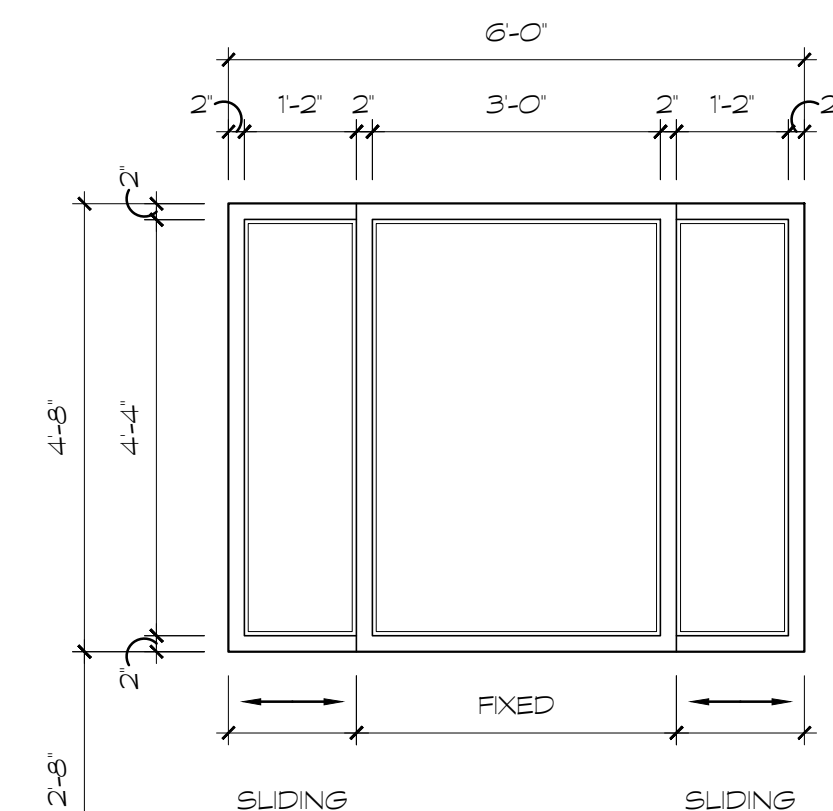
⑪ TYPICAL DOWNSPOUT BOOT DETAIL  
CONTRACTOR TO COORDINATE WITH STRUCTURAL FOR STEPS IN FOOTINGS. TYPICAL AT EACH DOWNSPOUT LOCATION.



⑧ ELEVATION  
SCALE: 1/8" = 1'-0"



⑩ SECTION  
SCALE: 1/8" = 1'-0"



⑫ WINDOW ELEVATION  
SCALE: 1/2" = 1'-0"

PROJECT NO.:  
17-008  
DATE  
04/07/17

REVISIONS  
NO. DATE  
POST BID  
ADDENDUM  
05/22/17

FACILITY CODE  
0000

K R H ARCHITECTS  
855 ABBOT RD - SUITE 4, DALTON, GA 30721  
TEL: (706) 529-5895

A New VISITOR'S PRESSBOX for:  
CARTERSVILLE HIGH SCHOOL  
320 E. CHURCH ST., CARTERSVILLE, GA 30120  
CARTERSVILLE CITY SCHOOLS

STATE OF GEORGIA  
KENNETH R. HABLESS  
REGISTERED ARCHITECT  
EXPIRATION DATE 12/31/2019

SHEET INDEX  
PRESSBOX  
ELEVATIONS,  
SECTIONS &  
DETAILS

DRAWING NO.:  
A1.2



DESIGN:

BUILDING CODE: INTERNATIONAL BUILDING CODE 2012 (IBC) WITH GEORGIA AMENDMENTS.

WIND:

120 MPH (3-SECOND GUST)  
EXPOSURE CATEGORY C  
COMPONENTS AND CLADDING: COMPONENTS AND CLADDING ELEMENTS NOT SPECIFICALLY DESIGNED ON THESE DRAWINGS SHALL BE DESIGNED ACCORDING TO THE WIND PRESSURES STIPULATED BY IBC 2012 FOR THE TRIBUTARY AREA OF THE SPECIFIC COMPONENT.

MIN ALLOWABLE DESIGN PRESSURE = 23.8 PSF (WALLS, 100 SQ FT, NON-END ZONE)

INTERNAL PRESSURE COEFFICIENT (GCp) = +0.18, -0.18

SEISMIC:

RISK CATEGORY III  
Is = 1.25 Is = 1.0  
Ss = 0.263 S1 = 0.103  
Sds = 0.279 Sd1 = 0.164  
SITE CLASS = D  
SEISMIC DESIGN CATEGORY = C  
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

SEISMIC RESISTING SYSTEM:

INTERMEDIATE REINFORCED MASONRY SHEARWALLS  
R = 3.5 Rb = 2.5 Cd = 2.25

BASE SHEAR N/S = 20 KIPS

SNOW:

GROUND SNOW LOAD = 5 PSF  
Is = 1.0  
FLAT ROOF SNOW LOAD = 5 PSF  
SNOW EXPOSURE FACTOR Ce = 1.0 SNOW THERMAL FACTOR Ct = 1.0

SHEET INDEX:

SO.1 GENERAL NOTES  
S1.1 PRESSBOX PLANS  
S2.1 SECTIONS & DETAILS  
S3.1 TYPICAL SECTIONS & DETAILS

MISCELLANEOUS:

- THE FOLLOWING NOTES APPLY TO ALL PROJECT RELATED STRUCTURAL DRAWINGS. THIS INCLUDES THESE DRAWINGS, FIELD SKETCHES AND RESPONSES TO REQUESTS FOR INFORMATION (RFIs), UNLESS OTHERWISE INDICATED.
- THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- STRUCTURAL DRAWINGS SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING PERTINENT ASPECTS OF ALL DISCREPANCIES OR OMISSIONS.
- NO OPENINGS OR MODIFICATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE PRIOR WRITTEN APPROVAL OF THE ARCHITECT.
- NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE PRIOR WRITTEN APPROVAL OF THE ARCHITECT.
- THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL DESIGN, ADEQUACY, SAFETY AND STABILITY OF TEMPORARY BRACING AND SHORINGS THAT MAY BE REQUIRED AS A RESULT OF THE CONTRACTORS CONSTRUCTION METHODS AND/OR SEQUENCES. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED ON THE STRUCTURAL FRAMING. APPLIED CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF ANY STRUCTURAL BUILDING ELEMENT.
- THE CONTRACTORS CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION LIFE CYCLE.
- DO NOT SCALE THESE DRAWINGS; USE DIMENSIONS, FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS; SEE ARCHITECTURAL DRAWINGS.
- THE CONTRACTOR SHALL INFORM THE PROFESSIONAL OF RECORD IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY OF SUCH DEVIATION BY THE PROFESSIONAL OF RECORD. REVIEW OF SHOP DRAWINGS, PRODUCT DATA, ETC. UNLESS THE CONTRACTOR HAS SPECIFICALLY INFORMED THE PROFESSIONAL OF RECORD OF SUCH DEVIATION AT THE TIME OF SUBMISSION AND THE ARCHITECT HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
- WHERE A SECTION OR DETAIL IS CUT ON THE PLAN, IT IS UNDERSTOOD TO BE REPRESENTATIVE OF ALL LIKE OR SIMILAR CONDITIONS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING SUCH REQUIREMENTS INTO THEIR SHOP DRAWINGS AND WORK.
- AT ALL TIMES THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONDITIONS OF THE JOBSITE INCLUDING SAFETY OF PERSONS AND PROPERTY. THE ARCHITECTS OR ENGINEERS PRESENCE AT THE JOB SITE OR REVIEW OF WORK DOES NOT IMPLY CONFIRMATION OF THE ADEQUACY OF THE CONTRACTORS MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR THE COMPLIANCE WITH OSHA REGULATIONS.
- CONSULT ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR LOCATION, SIZES, AND EXTENT OF CHASES, INSERTS, RECESSES, RIDGES, FINISHES, DEPRESSIONS, ETC., NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF RECORD IN WRITING OF ALL CONDITIONS ENCOUNTERED IN THE FIELD THAT ARE CONTRADICTORY TO THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
- STRUCTURAL CONTRACT DOCUMENTS SHALL NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR ANY MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR OR SUBCONTRACTOR.
- REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AND PUBLISHED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
- PRINCIPAL OPENINGS THROUGH THE FRAMING ARE SHOWN ON THESE DRAWINGS. OPENINGS 1'-4" IN WIDTH OR LENGTH (AND LESS) ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. THE GENERAL CONTRACTOR SHALL EXAMINE THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ALL ALL REQUIRED OPENINGS. ALL MECHANICAL OPENING LOCATIONS, UNIT OPERATING WEIGHTS, AND SIZES SHALL BE VERIFIED WITH THE MECHANICAL CONTRACTOR PRIOR TO FABRICATION. ANY DEVIATION FROM THE OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ENGINEERS ATTENTION FOR APPROVAL.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES IN ORDER TO COMPLY WITH THE CONTRACT DRAWINGS AND SPECIFICATIONS.

SUBMITTALS:

- STRUCTURAL DRAWINGS GIVE REPRESENTATIVE DETAILS AND ARE NOT INTENDED TO SHOW ALL CONDITIONS THAT MAY BE PRESENT. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH THE SPECIFIC REQUIREMENTS AS INDICATED IN THE PROJECT DOCUMENTS.
- CONTRACTOR SHALL SUBMIT A SCHEDULE OF SHOP DRAWING SUBMITTAL DATES TO ARCHITECT AT LEAST 30 DAYS PRIOR TO FIRST SUBMITTAL. FAILURE TO SUBMIT DRAWINGS ON DESIGNATED DATE MAY IMPACT REVIEW SCHEDULE.
- ANY MATERIALS OR PRODUCTS SUBMITTED FOR APPROVAL THAT ARE DIFFERENT FROM THE MATERIALS OR PRODUCTS SPECIFIED IN THE STRUCTURAL CONTRACT DOCUMENTS WILL BE CONSIDERED ONLY IF THE FOLLOWING CRITERIA ARE SATISFIED:
  - A COST SAVINGS TO THE OWNER IS DOCUMENTED AND SUBMITTED WITH THE REQUEST.
  - THE MATERIAL OR PRODUCT HAS BEEN APPROVED BY THE INTERNATIONAL CODE COUNCIL (ICC) AND THE ICC-ES REPORT IS SUBMITTED WITH THE REQUEST.SUBMITTALS NOT SATISFYING THE ABOVE CRITERIA WILL NOT BE CONSIDERED.
- REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER OF RECORD DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL TO THE STRUCTURAL ENGINEER OF RECORD. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
- COMPLETE SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL FABRICATED AND SPECIALTY BUILDING COMPONENTS INCLUDING (BUT NOT LIMITED TO) STAIR SYSTEMS WINDOW SYSTEMS. SHOP DRAWINGS SHALL BE SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF GEORGIA.
- ALL APPROVED SUBMITTALS, INCLUDING BUT NOT LIMITED TO SHOP DRAWINGS, SHALL BE MADE AVAILABLE ON THE JOBSITE FOR REVIEW BY THE INSPECTOR.
- REPRODUCTION OF CONTRACT DOCUMENTS FOR USE AS SHOP DRAWINGS IS NOT PERMITTED.
- THE CONTRACTOR SHALL MAINTAIN RECORDS AND RECORD ALL CHANGES MADE ON A SET OF CONTRACT STRUCTURAL DRAWINGS DURING THE PROGRESS OF THE WORK. THE IN-PROGRESS 'RECORD DRAWINGS' SHALL BE MAINTAINED IN THE CONSTRUCTION OFFICE AT THE SITE AND BE CLEARLY MARKED 'RECORD DRAWINGS'. 'RECORD DRAWINGS' SHALL BE UP-TO-DATE AND AVAILABLE FOR USE AT ANY TIME BY THE ENGINEER OR ARCHITECT. THE ENGINEER SHALL FURNISH ELECTRONIC FILES IN AUTOCAD FORMAT OF THE CONTRACT STRUCTURAL DRAWINGS TO THE CONTRACTOR FOR THE CONTRACTORS USE IN PREPARING A FINAL ELECTRONIC COPY OF THE 'RECORD DRAWINGS'. FINAL ELECTRONIC 'RECORD DRAWINGS' SHALL INCLUDE ALL CHANGES MADE INCLUDING PROJECT ADDENDA AND CHANGE ORDER MODIFICATIONS. ALL CHANGES SHALL BE IDENTIFIED WITH REVISION CLOUDS AND APPROPRIATE REVISION TRIANGLES. SUBMITTAL FOR ELECTRONIC 'RECORD DRAWINGS' SHALL BE MADE ON COMPACT DISK IN AUTOCAD FORMAT ALONG WITH ONE FULL SIZE SET OF BOND PLOTS TO THE ENGINEER. PLOTS SHALL BE GENERATED FROM THE COMPACT DISK OF ELECTRONIC FILES. CONTRACTOR SHALL MAINTAIN ELECTRONIC FILE NAMES AND PLOT SHEET NUMBERING PURSUANT TO THE ORIGINAL CONTRACT DOCUMENT FORMAT.

FOUNDATIONS:

- SPREAD FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUSTAINING AN ASSUMED NET ALLOWABLE BEARING PRESSURE OF 15 KSF FOR INDIVIDUAL COLUMN FOOTINGS UNDER FULL SERVICE LIVE AND DEAD LOAD.
- THE SITE SHALL BE PREPARED IN ACCORDANCE WITH CIVIL DRAWINGS AND PROJECT SPECIFICATIONS. A GEOTECHNICAL INVESTIGATION HAS NOT BEEN PERFORMED ON THIS SITE PRIOR TO THE ISSUANCE OF THESE DRAWINGS. A QUALIFIED GEOTECHNICAL ENGINEER SHALL VERIFY ALL ASSUMPTIONS AND REPORT ANY VARIATIONS OR DISCREPANCIES TO THE ENGINEER.
- THE FOOTINGS HAVE BEEN POSITIONED AT THE ESTIMATED ELEVATION WHICH WILL PROVIDE SUITABLE BEARING. HOWEVER, IF ADEQUATE BEARING CAPACITY IS NON-EXISTENT AT THESE ESTIMATED ELEVATIONS, THE FOOTING SHALL BE LOWERED TO AN ELEVATION WHERE THE PRESCRIBED SAFE BEARING CAPACITY EXISTS (AS RECOMMENDED BY A QUALIFIED GEOTECHNICAL ENGINEER).
- FOOTINGS MAY BE CAST INTO AN EARTH-FORMED TRENCH IF SOIL CONDITIONS PERMIT.
- EXCAVATION FOR FOOTINGS SHALL BE CUT TO ACCURATE SIZE AND DIMENSIONS AS SHOWN ON PLANS. ALL SOIL BELOW SLABS AND FOOTINGS SHALL BE PROPERLY COMPACTED AND SUBGRADE BROUGHT TO A REASONABLE TRUE AND LEVEL PLANE BEFORE PLACING CONCRETE.
- IN AREA OF THE BUILDING, EXISTING ORGANIC MATERIAL, UNSUITABLE SOIL, ABANDONED FOOTINGS AND ANY OTHER EXISTING UNSUITABLE MATERIALS SHALL BE REMOVED. ANY CUT AND FILL REQUIREMENTS SPECIFIED BY CIVIL SHALL BE AS INSTALLED PURSUANT TO THE GEOTECHNICAL REPORT NOTED IN ITEM 2 OF THIS SECTION.
- FOOTING CONCRETE SHALL BE CAST ON THE SAME DAY THE EXCAVATION IS APPROVED. IF THE BEARING SURFACE IS ALLOWED TO BECOME DISTURBED IN ANY WAY, IT SHALL BE REWORKED TO THE SATISFACTION OF AN INDEPENDENT TESTING AGENCY PRIOR TO CASTINGS OF THE CONCRETE.
- ALL EXCAVATIONS AND STRUCTURE BEARING PADS SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY PRIOR TO CONCRETE PLACEMENT. THE INDEPENDENT TESTING AGENCY SHALL BE THE SOLE JUDGE AS TO THE SUITABILITY OF THE BEARING MATERIAL.
- BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 1'-6" BELOW FINAL GRADE FOR FROST PROTECTION.
- NO EXCAVATION SHALL BE CLOSER THAN AT A SLOPE OF 21 (2 HORIZONTAL TO 1 VERTICAL) TO A FOOTING. PROVIDE SHORINGS AND PROTECTION FOR EXCAVATION BANKS AS NECESSARY TO PRESERVE SAFETY AND PREVENT CAVING.
- ALL BEARING STRATA SHALL BE ADEQUATELY DRAINED BEFORE FOUNDATION CONCRETE IS PLACED.
- BACKFILL AGAINST WALLS SHALL BE PLACED IN 8" LIFTS AND SHALL BE DEPOSITED EVENLY AGAINST EACH SIDE OF WALL UNTIL THE LOWER FINAL GRADE IS REACHED. BACKFILL SHALL NOT BE PLACED AGAINST WALLS DEPENDENT UPON TOP AND BOTTOM SLABS/FOUNDATION FOR SUPPORT UNTL SUCH SLABS HAVE ATTAINED MINIMUM SUFFICIENT BRACING AND SHORING FOR ALL WORK DURING THE CONSTRUCTION PROCESS. RETAINING WALLS ARE NOT DESIGNED TO CANTILEVER AT ANY TIME UNLESS EXPLICITLY NOTED ON DRAWINGS.
- THE CONTRACTOR SHALL PROVIDE AN ADEQUATE DRAINAGE SYSTEM FOR ALL BACKFILL CONDITIONS PER CIVIL AND ARCHITECTURAL DRAWINGS AND SPECIFICATIONS.
- THERE SHALL BE NO HORIZONTAL CONSTRUCTION JOINTS IN ANY FOOTING WITHOUT PRIOR WRITTEN APPROVAL FROM ENGINEER.

CONCRETE:

- ALL CONCRETE DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 318-11 AND ACI 308-10.
- CEMENT USED SHALL BE TYPE I OR II CONFORMING TO ASTM C-150. CONCRETE SHALL DEVELOP A MINIMUM 28 DAY STRENGTH AND DENSITY AS FOLLOWS:

FOOTINGS ELEVATED SLAB	STRENGTH (PSI) 3000 3000	DENSITY (PCF) 145 - 150 145 - 150
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- AGGREGATE SHALL BE WELL GRADATED AND SHALL CONFORM TO THE FOLLOWING:

FOOTINGS	1 COARSE AGGREGATE (ASTM C-33)
ELEVATED SLAB	2 COARSE AGGREGATE (ASTM C-330)
- CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS FOR REVIEW IN ADVANCE OF CONCRETE PLACEMENT. CONCRETE MIX DESIGN SHALL INCLUDE ALL STRENGTH DATA NECESSARY TO SHOW COMPLIANCE WITH THE PROJECT SPECIFICATIONS BY EITHER THE TRIAL BATCH OR FIELD EXPERIENCE METHOD AND SHALL BE CERTIFIED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA. RESULTS OF ALL COMPRESSIVE STRENGTH TEST SHALL BE MADE AVAILABLE AT THE JOB SITE FOR REVIEW BY THE INSPECTOR.
- ALL MIXING, TRANSPORTING, PLACING AND CURING OF CONCRETE SHALL BE DONE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE.
- NO ADDITIONAL WATER SHALL BE ADDED TO CONCRETE AT THE JOB SITE.
- MINIMUM CONCRETE COVER UNLESS NOTED OTHERWISE:
  - #11 BARS AND SMALLER: 3/4 INCHES
  - UNFORMED SURFACE IN CONTACT WITH THE GROUND: 3 INCHES
  - BASEMENT WALLS: 2 INCHES EXTERIOR  
3/4 INCHES INTERIOR
  - FORMED SURFACES EXPOSED TO EARTH OR WEATHER:  
#6 BARS AND LARGER: 2 INCHES  
#5 BARS AND SMALLER: 1 1/2 INCHES
  - FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER:  
BEAMS, GIRDERS AND COLUMNS: 1 1/2 INCHES  
SLABS, WALLS, AND JOISTS: 3/4 INCHES
- PLACEMENT OF CONCRETE, COLD WEATHER AND HOT WEATHER PRECAUTIONS, MATERIAL AND PROPORTIONING REQUIREMENTS, REBAR COVER AND DETAILING SHALL CONFORM TO REQUIREMENTS OF THE AMERICAN CONCRETE INSTITUTE (ACI) 318-11.
- REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS AND SPECIFICATIONS FOR SLAB FINISHES, SLAB DEPRESSIONS, ELEVATIONS AND ENCASED OR EMBEDDED ITEMS.
- PIPES AND CONDUITS EMBEDDED IN CONCRETE SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
  - NO MATERIAL HARMFUL TO CONCRETE (SUCH AS , BUT NOT LIMITED TO, ALUMINUM) IS PERMITTED.
  - NO EMBEDMENT OR PENETRATION WHICH IMPAIRS THE STRUCTURAL STRENGTH OR INTEGRITY IS PERMITTED.
  - CONDUITS AND PIPES SHALL NOT HAVE A DIAMETER THAT EXCEEDS 1/3 THE OVERALL THICKNESS OF THE STRUCTURAL ELEMENT IN WHICH THEY ARE EMBEDDED.
  - MINIMUM CENTER TO CENTER SPACING SHALL NOT BE CLOSER THAN 3 DIAMETERS OR WIDTHS.
  - PLACEMENT SHALL OCCUR ABOVE BOTTOM LAYER OF REINFORCEMENT AND BELOW TOP LAYER OF REINFORCEMENT AND SHALL NOT CAUSE REINFORCEMENT TO BE CUT, BENT OR DISPLACED IN ANY MANNER.
  - PLACEMENT SHALL MAINTAIN A MINIMUM CLEARANCE FROM REINFORCEMENT OF 3 REINFORCING BAR DIAMETERS OR 3/4" FROM WELDED WIRE FABRIC REINFORCEMENT.
  - PLUMBING AND ELECTRICAL CONDUITS SHALL BE PLACED BELOW SLAB ON GRADE.
- FORMING SHALL BE OF WOOD, STEEL, OR FIBERGLASS OF SATISFACTORY QUALITY AND CONDITION.
- NO ADMIXTURES SHALL BE ADDED TO THE CONCRETE UNLESS APPROVED BY THE ENGINEER.
- REINFORCING SHALL CONFORM TO ASTM A615, GR60 UNLESS NOTED OTHERWISE.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 GRADE 60.
- REINFORCING STEEL AND ACCESSORIES SHALL BE DETAILED IN ACCORDANCE WITH ACI 315 (MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES) AND CRSI MSP-1 (MANUAL OF STANDARD PRACTICE), LATEST EDITION.
- ALL 'CONTINUOUS' REINFORCEMENT SHALL HAVE MINIMUM LAP OF 'B' TYPE (ACI 318-11, SECTION 12.15.1) AT SPICES UNLESS NOTED OTHERWISE.
- PROVIDE REINFORCING CHAIRS FOR ALL SLAB-ON-GRADE REINFORCING.
- SUBMIT REINFORCING PLACEMENT AND DETAIL (SHOP) DRAWINGS FOR REVIEW. NO REINFORCING BARS SHALL BE INSTALLED UNTIL THE SHOP DRAWINGS HAVE BEEN REVIEWED AND RETURNED.
- ALL REINFORCING SHALL BE SUPPORTED IN FORMS SPACED WITH NECESSARY ACCESSORIES AND SHALL BE SECURELY WIRED TOGETHER IN ACCORDANCE WITH CRSI 'MANUAL OF STANDARD PRACTICE' (LATEST EDITION).
- ALL REINFORCING SPICES SHALL BE CONSIDERED CLASS 'B' UNLESS NOTED OTHERWISE IN PLAN OR SECTION.
- WHERE WELDED WIRE FABRIC REINFORCEMENT IS SPECIFIED IN SLABS ON GRADE PLACEMENT SHALL BE 1" BELOW TOP OF SLAB, OVERLAP EACH REINFORCING SHEET TWO FULL PANELS AND TIE CROSS WIRES ON EACH SIDE.
- SCHEDULED OR DETAILED REINFORCING STEEL SHALL NOT BE TACK WELDED FOR ANY REASON. WELDED REINFORCING STEEL AND/OR SPICES ARE PERMITTED ONLY WHERE SHOWN ON DRAWINGS. WHERE WELDING IS PERMITTED IT SHALL CONFORM TO AWS D1.4, STRUCTURAL WELDING CODE - REINFORCING STEEL.
- BASE PLATES, ANCHOR RODS, SUPPORT ANGLES, ETC. BELOW GRADE SHALL BE COVERED WITH A MINIMUM OF 4" OF CONCRETE.
- WHERE DOVELS, BOLTS OR INSERTS ARE CALLED TO BE ANCHORED TO CAST IN PLACE OR PRECAST CONCRETE ELEMENTS USING EPOXY ADHESIVES, USE ANCHORAGE SYSTEM EQUAL TO HILTI HIT DOVELING (HY-200). FOLLOW ALL MANUFACTURERS RECOMMENDATIONS. ALTERNATE ANCHORAGE SYSTEMS MAY BE PERMITTED WITH STRUCTURAL ENGINEERS APPROVAL.
- RESULTS FOR ALL CONCRETE COMPRESSIVE STRENGTH TEST SHALL BE AVAILABLE ON THE JOB SITE FOR REVIEW BY THE INSPECTOR.

MASONRY:

- ALL MASONRY DESIGN AND CONSTRUCTION SHALL CONFORM TO ACI 530-11 AND ACI 530.1-11.
- MASONRY SHALL BE MEDIUMWEIGHT AND HAVE A MINIMUM COMPRESSIVE STRENGTH, fm, OF 1500 PSI BASED ON GROSS AREA. MORTAR SHALL CONFORM TO ASTM C270 TYPE S OR M. GROUT SHALL CONFORM TO ASTM C476, WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI.
- REINFORCING BARS SHALL CONFORM TO ASTM A 615 GRADE 60 UNLESS NOTED OTHERWISE.
- CONTINUOUS WIRE REINFORCING (JOINT REINFORCING) SHALL BE GALVANIZED LADDER TYPE FABRICATED UNITS WITH A SINGLE PAIR OF 3/8" DIAMETER SIDE RODS AND CROSS RODS (EXTRA HEAVY) FABRICATED FROM COLD DRAWN STEEL WIRE COMPLYING WITH ASTM A82. JOINT REINFORCING SHALL BE SPACED AT 8" O.C. VERTICALLY IN ALL MASONRY WALLS UNLESS NOTED OTHERWISE.
- VERTICAL CONTROL JOINTS IN MASONRY WALLS ARE NOT INDICATED ON THESE DRAWINGS. 'HORIZONTAL BOND BEAM' AND UNTEL REINFORCING SHALL BE CONTINUOUS ACROSS VERTICAL CONTROL JOINTS. HORIZONTAL JOINT REINFORCING (DUR-O-WALL) SHALL BE TERMINATED ON EITHER SIDE OF VERTICAL CONTROL JOINTS. WALLS SHORTER THAN 15'-0" IN LENGTH SHALL NOT HAVE VERTICAL CONTROL JOINTS.
  - AT EXTERIOR WALLS, SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF VERTICAL CONTROL JOINTS. JOINTS SHALL BE PLACED AT A SPACING NOT TO EXCEED 30'-0" ON CENTER. JOINTS SHALL NOT BE LOCATED CLOSER THAN 2'-6" TO THE JAMBS OF ANY EXTERIOR WALL OPENING. JOINTS SHALL NOT BE LOCATED FURTHER THAN 15'-0" FROM ANY CORNER, NOR CLOSER THAN 5'-0" FROM ANY CORNER.
  - AT INTERIOR SHEAR WALLS, JOINTS SHALL BE PLACED AT A SPACING NOT TO EXCEED 30'-0" ON CENTER. JOINTS SHALL NOT BE LOCATED CLOSER THAN 2'-6" TO THE JAMBS OF ANY SHEAR WALL OPENING. JOINTS SHALL NOT BE LOCATED FURTHER THAN 15'-0" FROM ANY CORNER, NOR CLOSER THAN 5'-0" FROM ANY CORNER.
  - AT INTERIOR NON-SHEAR WALLS, VERTICAL CONTROL JOINTS SHALL BE PLACED AT A SPACING NOT TO EXCEED 30'-0" ON CENTER. JOINTS SHALL BE LOCATED AT WALL JAMBS, WHERE PRACTICAL, AND SHALL STEP 6" HORIZONTALLY AT MASONRY UNTEL LOCATIONS. WHERE WALLS SIT ON TOP OF A CAST SLAB-ON-GRADE, ALIGN WALL CONTROL JOINTS WITH SLAB CONTROL JOINTS. JOINTS SHALL BE LOCATED AT ALL CORNER/TEE INTERSECTIONS WHERE THE LEGS OF EACH CORNER/TEE EXCEED 15'-0" IN LENGTH.
- MASONRY PILASTERS SHALL BE LOCATED ADJACENT TO CONTROL OR EXPANSION JOINTS PER TYPICAL DETAILS.
- ALL REINFORCED CELLS AND ALL CELLS BELOW FINISH FLOOR SHALL BE GROUTED SOLID.
- WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL BLOCK CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICAL. DOWELS MAY BE GROUTED INTO A CELL IN VERTICAL ALIGNMENT EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING.
- REINFORCING STEEL SHALL BE SECURED IN PLACE BEFORE GROUTING STARTS.
- VERTICAL BARS SHALL BE HELD IN POSITION WITH PRE-MANUFACTURED TIES AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 200 DIAMETERS OF THE REINFORCING NOR 10 FEET.
- VERTICAL CELLS THAT WILL BE GROUTED SHALL HAVE A VERTICAL ALIGNMENT TO MAINTAIN A CONTINUOUS UNOBSTRUCTED CELL AREA NOT LESS THAN 2'-1/2" x 3'.
- GROUTING SHALL BE STOPPED 1'-1/2" BELOW THE TOP OF A COURSE SO AS TO FORM A KEY AT THE JOINT.
- GROUTING OF MASONRY BEAMS OVER OPENINGS SHALL BE DONE IN ONE CONTINUOUS OPERATION.
- ALL BOLTS INSERTED IN THE WALLS SHALL BE GROUTED SOLIDLY INTO POSITION.
- WHERE EXPANSION BOLTS OR OTHER ANCHORS ARE EMBEDDED INTO THE SIDE OF MASONRY WALLS, THE CELLS SHALL BE FULLY GROUTED AT LEAST 8" ABOVE AND BELOW EACH BOLT OR ANCHOR.
- WHERE NOT OTHERWISE SHOWN, MASONRY WALL FOOTINGS SHALL BE 12" THICK AND HAVE A MINIMUM OF 4" PROJECTION ON EACH SIDE OF WALL. REINFORCE WITH (2) #5 BARS CONTINUOUS TOP AND BOTTOM.
- WALLS SHALL BE GROUTED USING LOW LIFT GROUTING TECHNIQUES.
- ALL MASONRY WALLS SHALL BE ASSUMED TO BE RUNNING BOND, UNLESS NOTED OTHERWISE IN PLAN OR SECTION.
- MASONRY MORTAR SHALL BE TYPE 'S' AND CONFORM TO ASTM C-270

PROJECT NO.:

17-008

DATE

04/07/17

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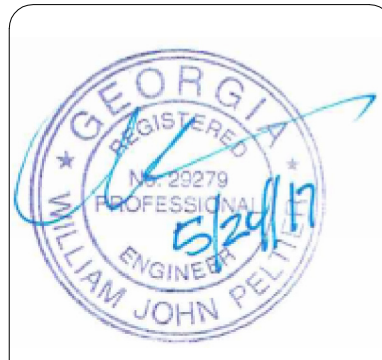
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K R H ARCHITECTS

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A New VISITOR'S PRESSBOX for:

CARTERSVILLE HIGH SCHOOL  
320 E. CHURCH ST., CARTERSVILLE, GA 30120  
CARTERSVILLE CITY SCHOOLS



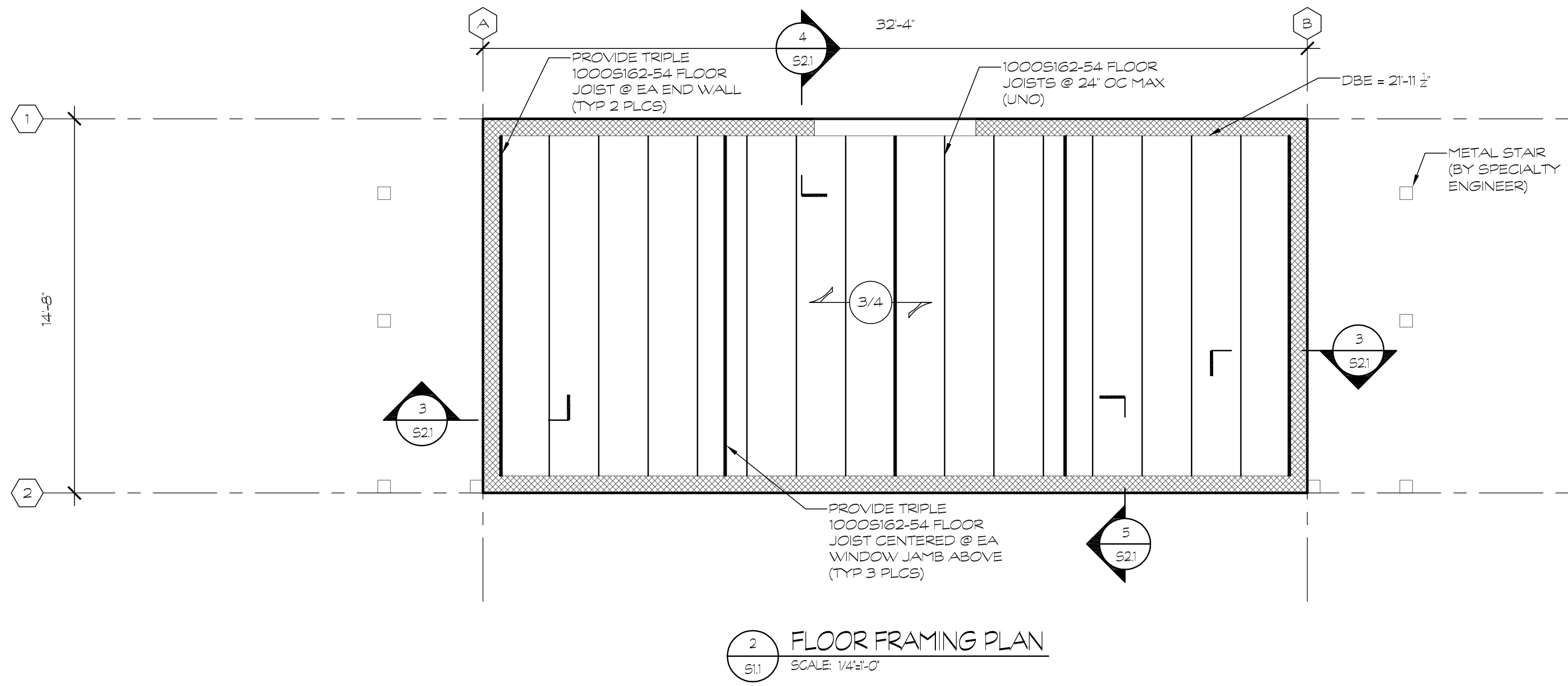
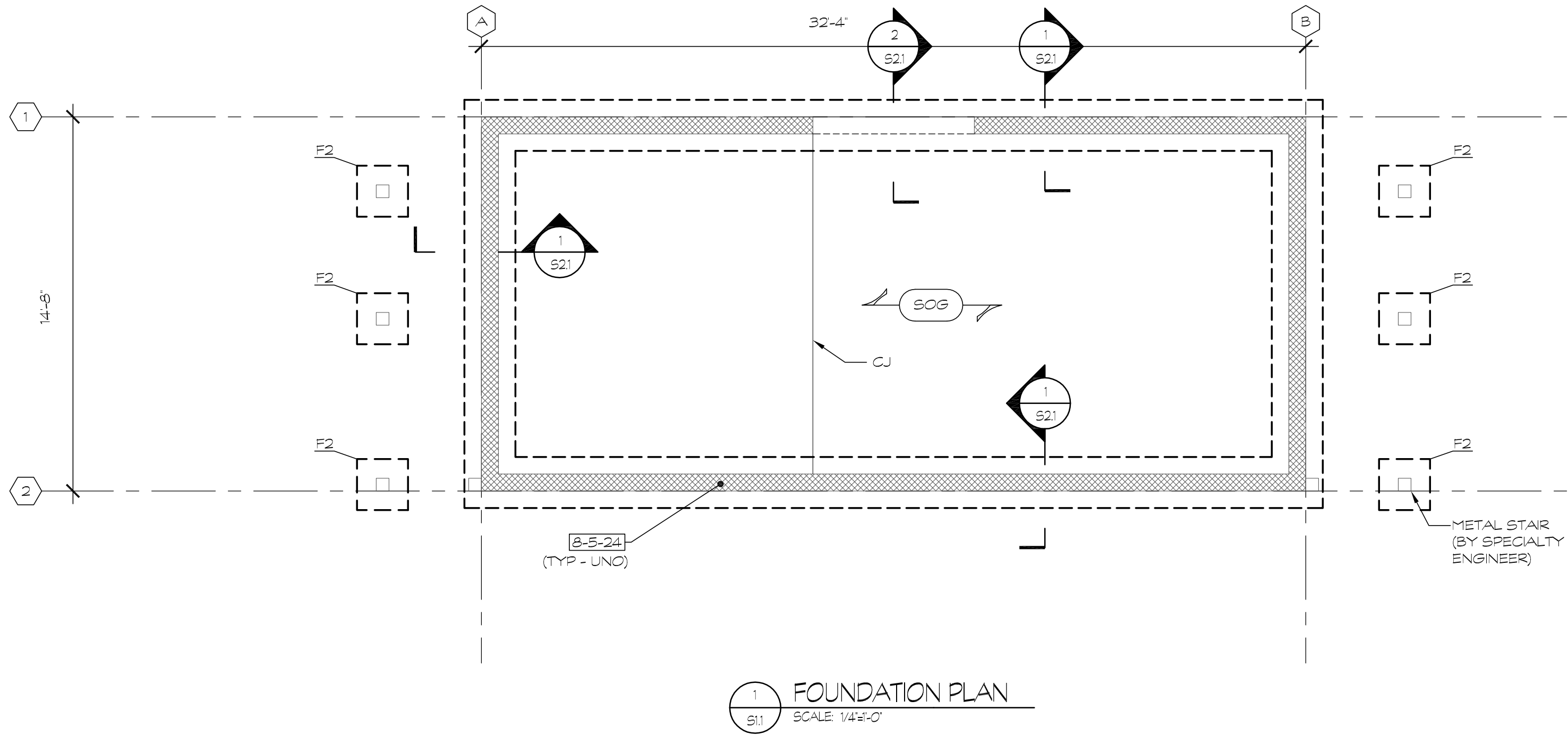
SHEET INDEX

GENERAL NOTES

DRAWING NO.:

SO.1





FLOOR FRAMING NOTES:

1. DBE INDICATES FLOOR DECK BEARING ELEVATION = SEE PLAN.
2. INDICATES SPAN OF 3/4 48/24 (5 PLY MIN) T&G PLYWOOD FLOOR DECKING. FASTEN TO JOISTS W/8 SCREWS @ 4' OC ON EDGE & 12' OC IN FIELD (TYP). STAGGER PLYWOOD SHEETS.
3. SEE ARCH DRAWINGS FOR ALL DIMENSIONS NOT SHOWN.

DEAD LOADS:  
PRESSBOX - 10 PSF  
STAIRS - 25 PSF MIN

LIVE LOADS:  
PRESSBOX - 50 PSF  
STAIRS - 100 PSF MIN

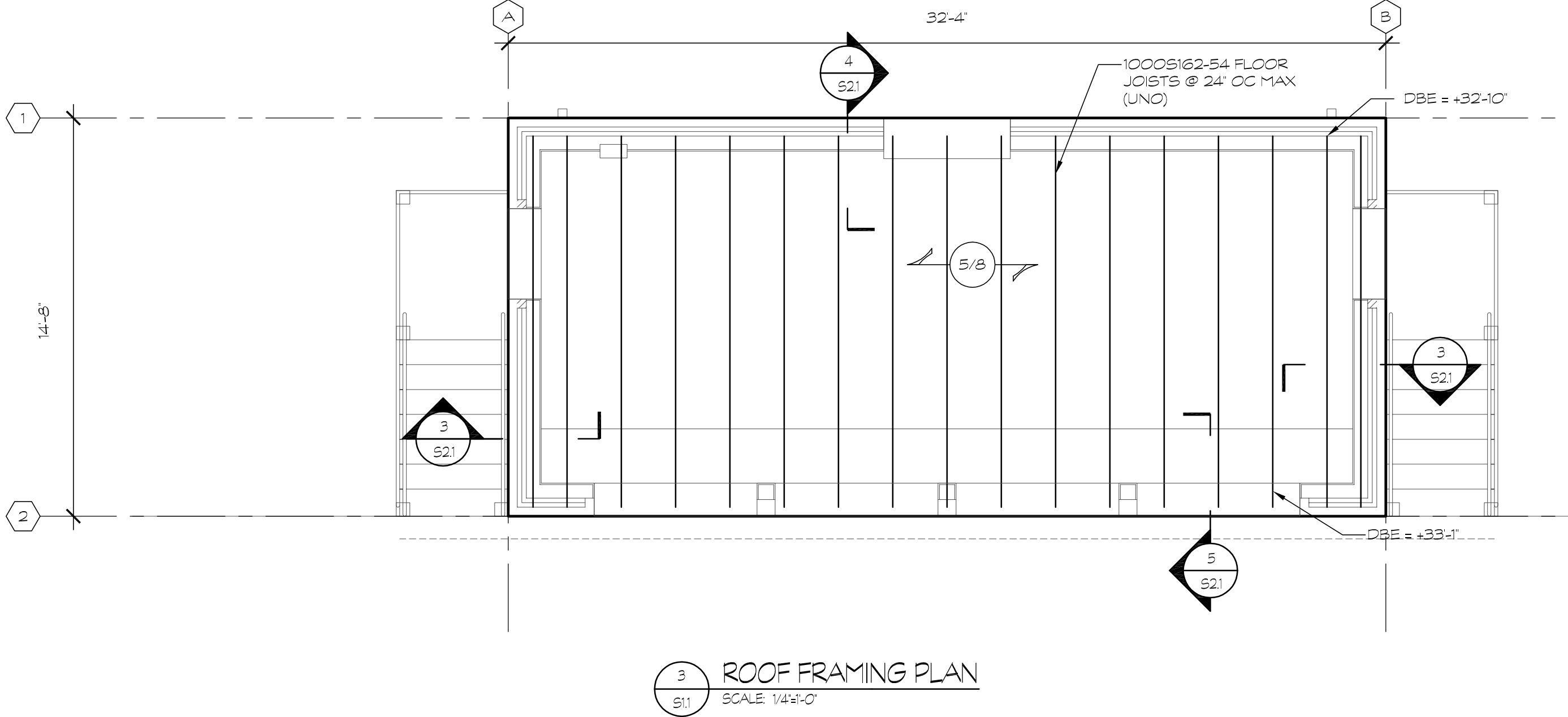
SLAB & FOUNDATION NOTES:

1. (N) SLAB ON GRADE SHALL BE 4" CONC SLAB (3000 PSI) ON 10 MIL (MIN) VAPOR RETARDER ON 4" AGGREGATE BASE COURSE STONE W/1" LAYER 6x6-W1.4x1.4 WWF 1" FROM TOP OF SLAB, UNO ON PLAN. FFE ± 0'-0"
2. INDICATES TOP OF FOOTING ELEVATION BELOW FFE.
4. INDICATES PARTIALLY GROUTED, MASONRY SHEARWALLS. SEE TYPICAL DETAILS ON S3.1 FOR ADDITIONAL INFORMATION.
5. INDICATES CMU WALL REINFORCEMENT. ALL REINFORCEMENT TO BE CENTERED IN CELLS, UNO. AT MINIMUM, ALL REINFORCED CELLS SHALL BE GROUTED SOLID. ALL MASONRY WALLS SHALL HAVE LADDER TYPE HORIZONTAL REINFORCING (MIN NO 9 SIDE RODS) AT 16" OC, UNO.  
 BAR SPACING (INCHES)  
 BAR SIZE  
 NOMINAL WALL SIZE (INCHES)
6. "CJ" INDICATES CONTROL/CONSTRUCTION JOINTS IN SLAB.
7. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS NOT SHOWN.

FOOTING SCHEDULE			
MARK	SIZE	REINFORCING	REMARKS
F2	2'-0" x 2'-0" x 1'-4"	(3) #5 BW BOT	

NOTES:

1. PROVIDE 3" COVER MIN FOR ALL REINFORCING.



ROOF FRAMING NOTES:

1. DBE INDICATES ROOF DECK BEARING ELEVATION = SEE PLAN.
2. INDICATES SPAN OF 5/8 40/20 PLYWOOD ROOF DECKING. FASTEN TO JOISTS W/8 SCREWS @ 4' OC ON EDGE & 12' OC IN FIELD (TYP). STAGGER PLYWOOD SHEETS.
3. SEE ARCH DRAWINGS FOR ALL DIMENSIONS NOT SHOWN.

ROOF DEAD LOAD: 10 PSF

ROOF LIVE LOAD: 20 PSF

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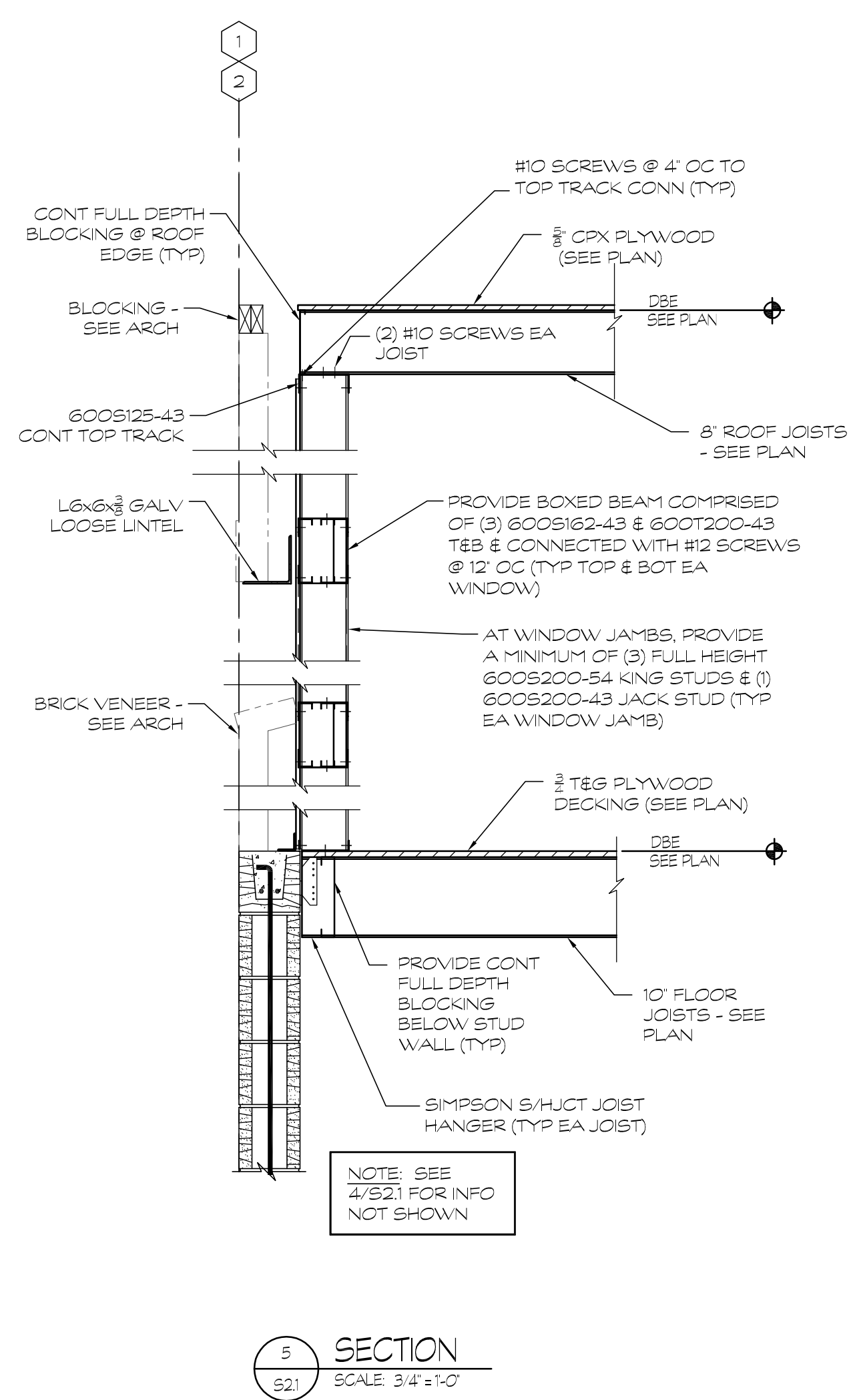
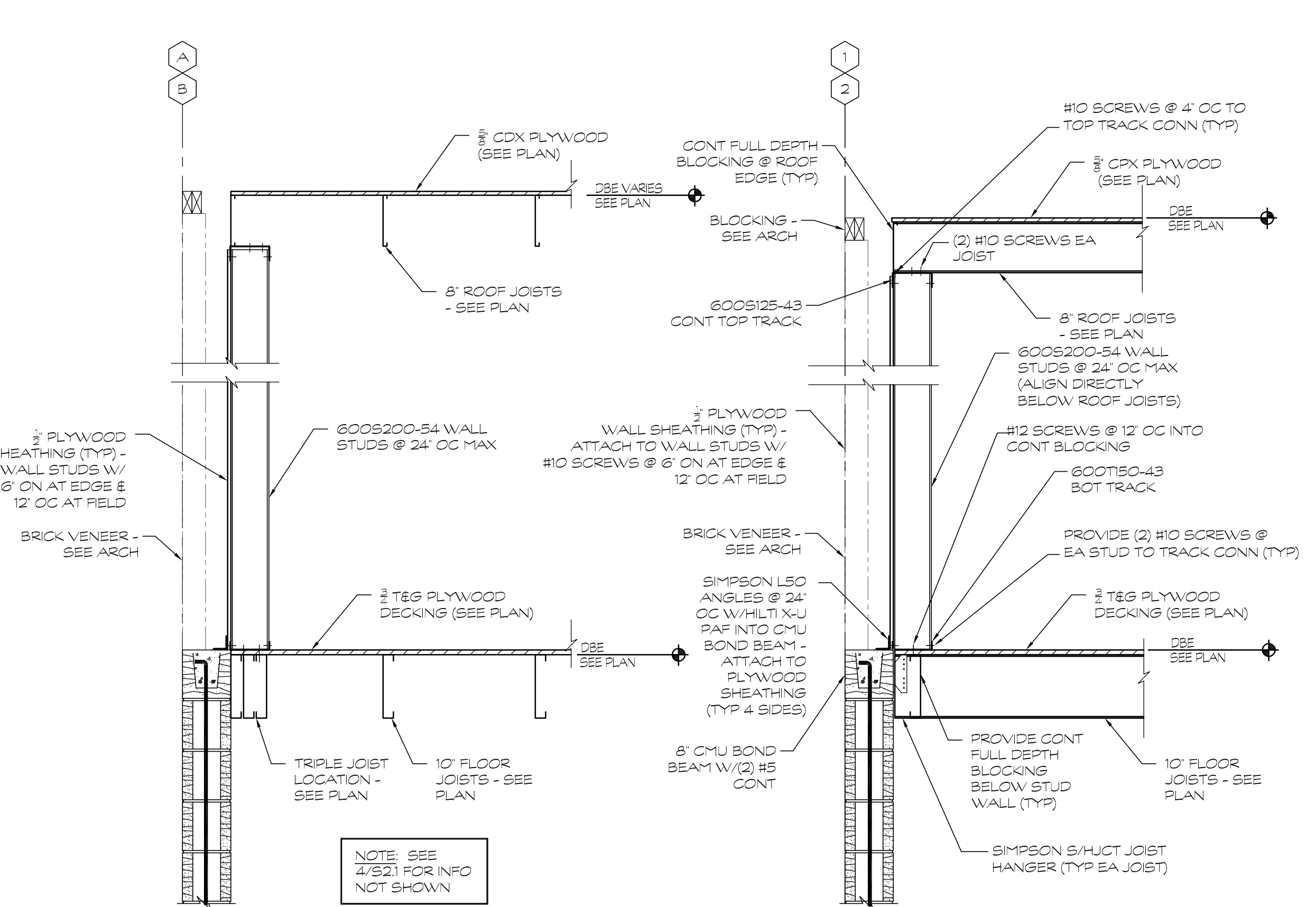
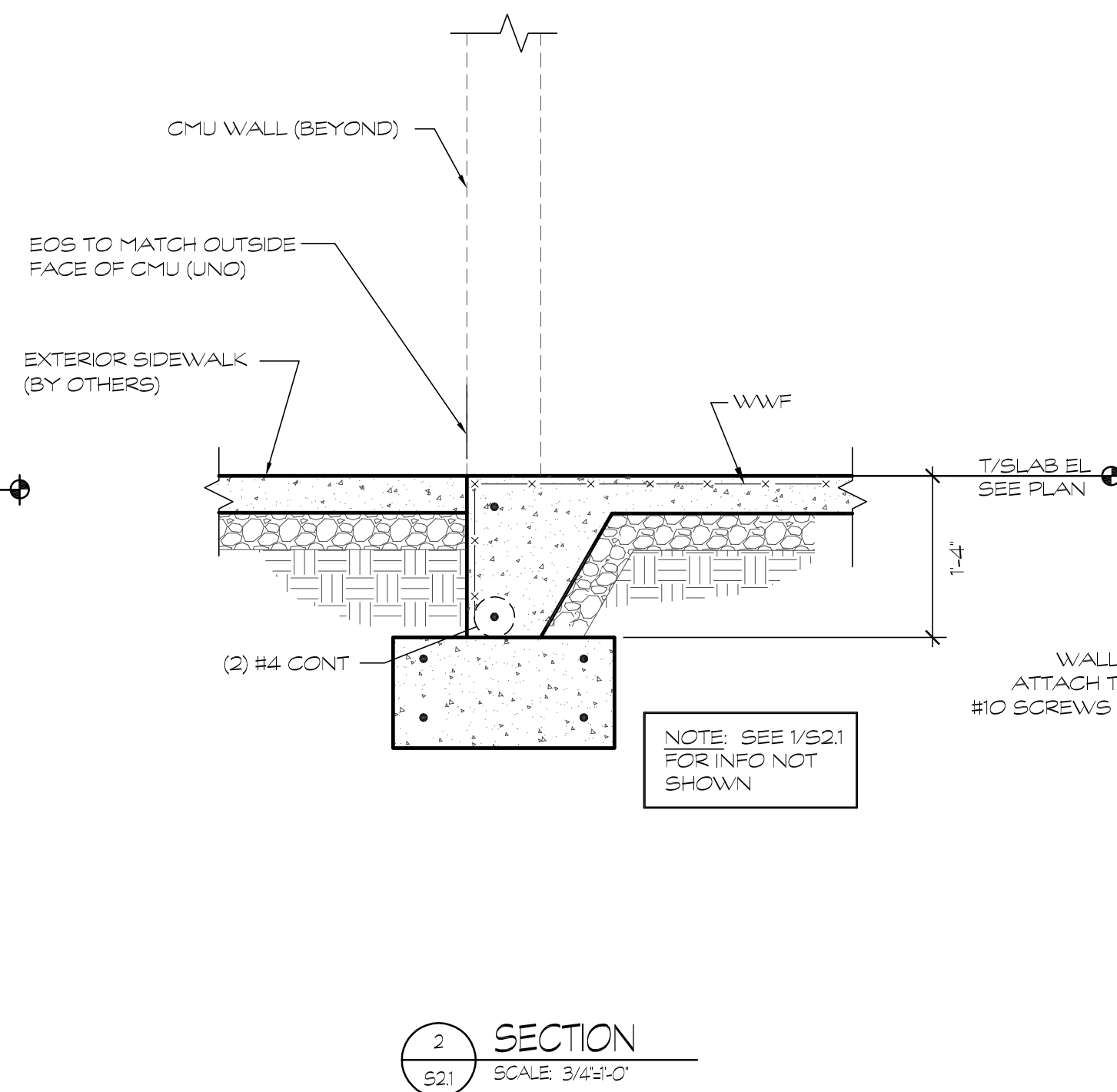
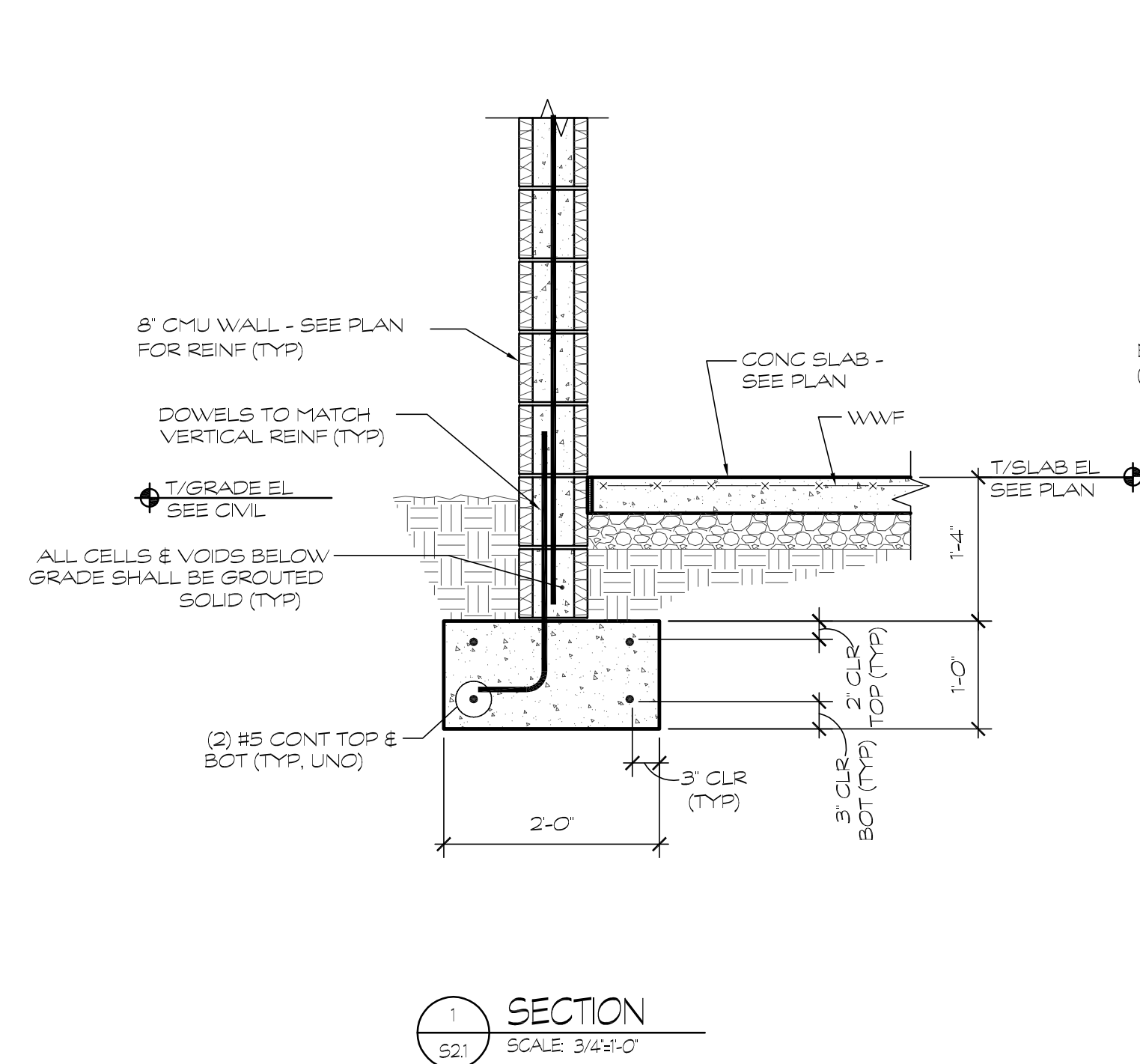
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PRESSBOX PLANS

DRAWING NO.:

S1.1





SECTION 5  
SCALE: 3/4" = 1'-0"

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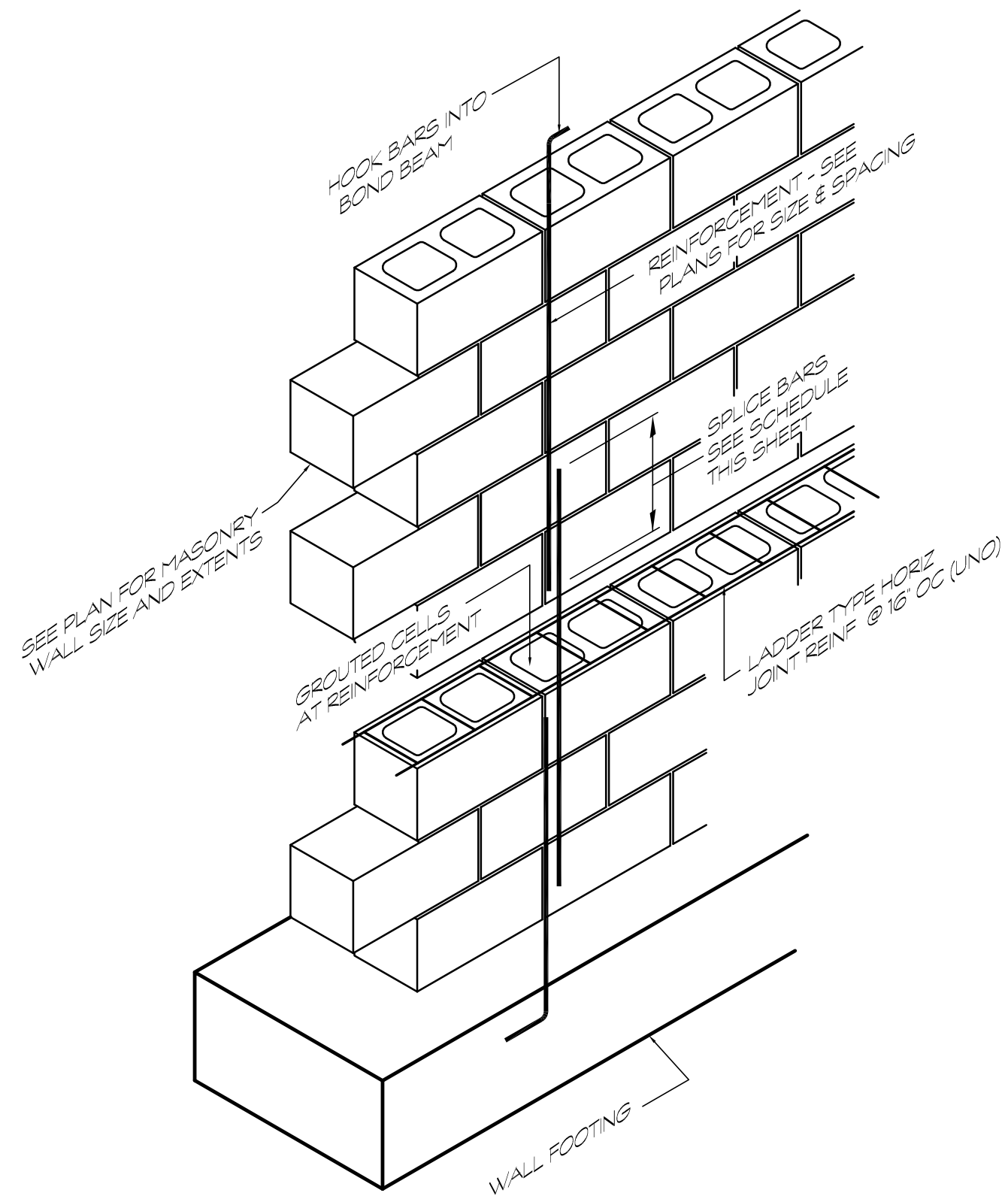
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SHEET INDEX  
SECTIONS &  
DETAILS

DRAWING NO.:  
**S2.1**



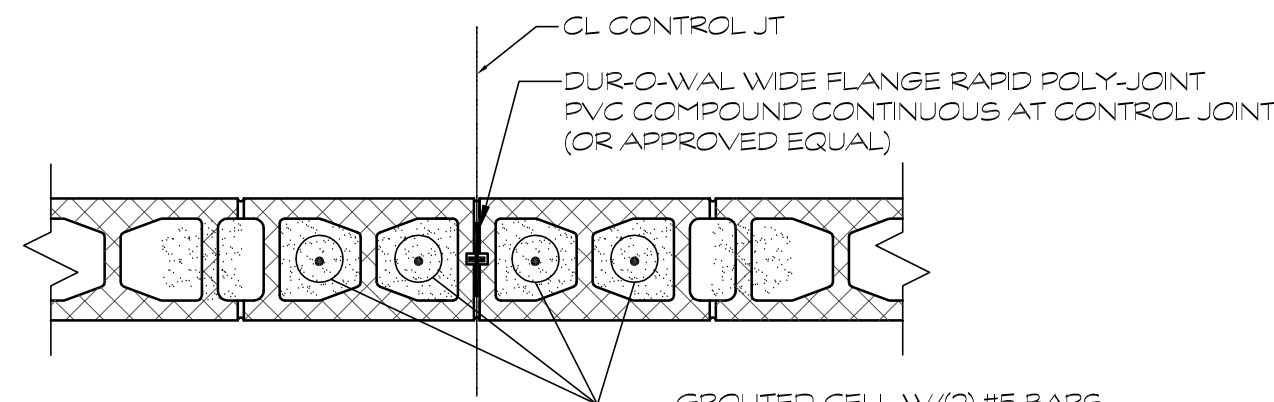


1 TYPICAL DETAIL OF LOW-LIFT REINFORCED MASONRY CONSTRUCTION  
SCALE: NTS

REINFORCING LAP LENGTH SCHEDULE*	
BAR SIZE	LAP LENGTH
#4	21"
#5	26" (8' CMU)
#6	43"

\* LAP LENGTHS APPLY TO 8" OR 12" CMU WITH REINFORCING CENTERED IN CELL (UNO).

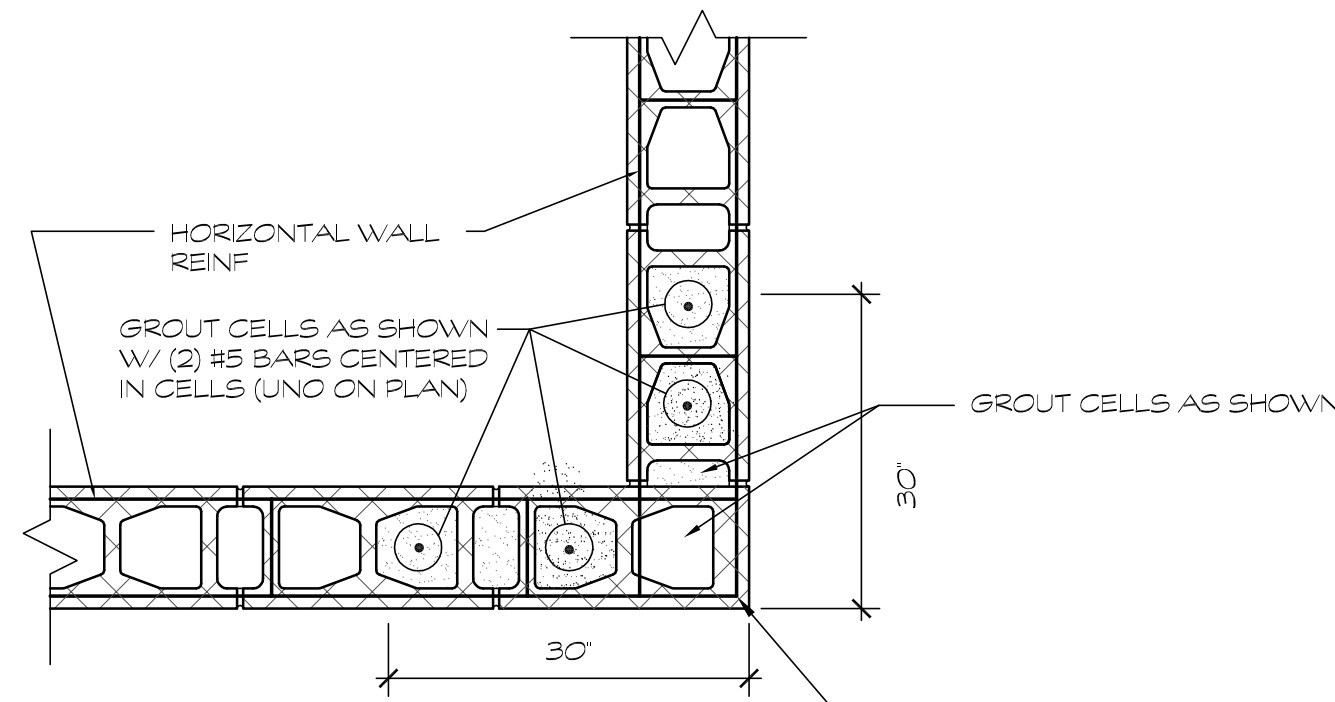
- LOW LIFT GROUTING PROCEDURE
1. CONSTRUCT WALL TO HEIGHT OF 5'-0". ALLOW MORTAR TO SET SUFFICIENTLY TO WITHSTAND GROUT PRESSURE.
  2. INSPECT UNITS FOR ALIGNMENT. CLEAN OUT CELLS TO BE FILLED.
  3. FILL CELLS TO 1 1/2" BELOW TOP COURSE.
  4. DELAY 3 TO 5 MINUTES PRIOR TO CONSOLIDATING TO ALLOW WATER TO BE ABSORBED BY MASONRY.



@ SHEARWALL

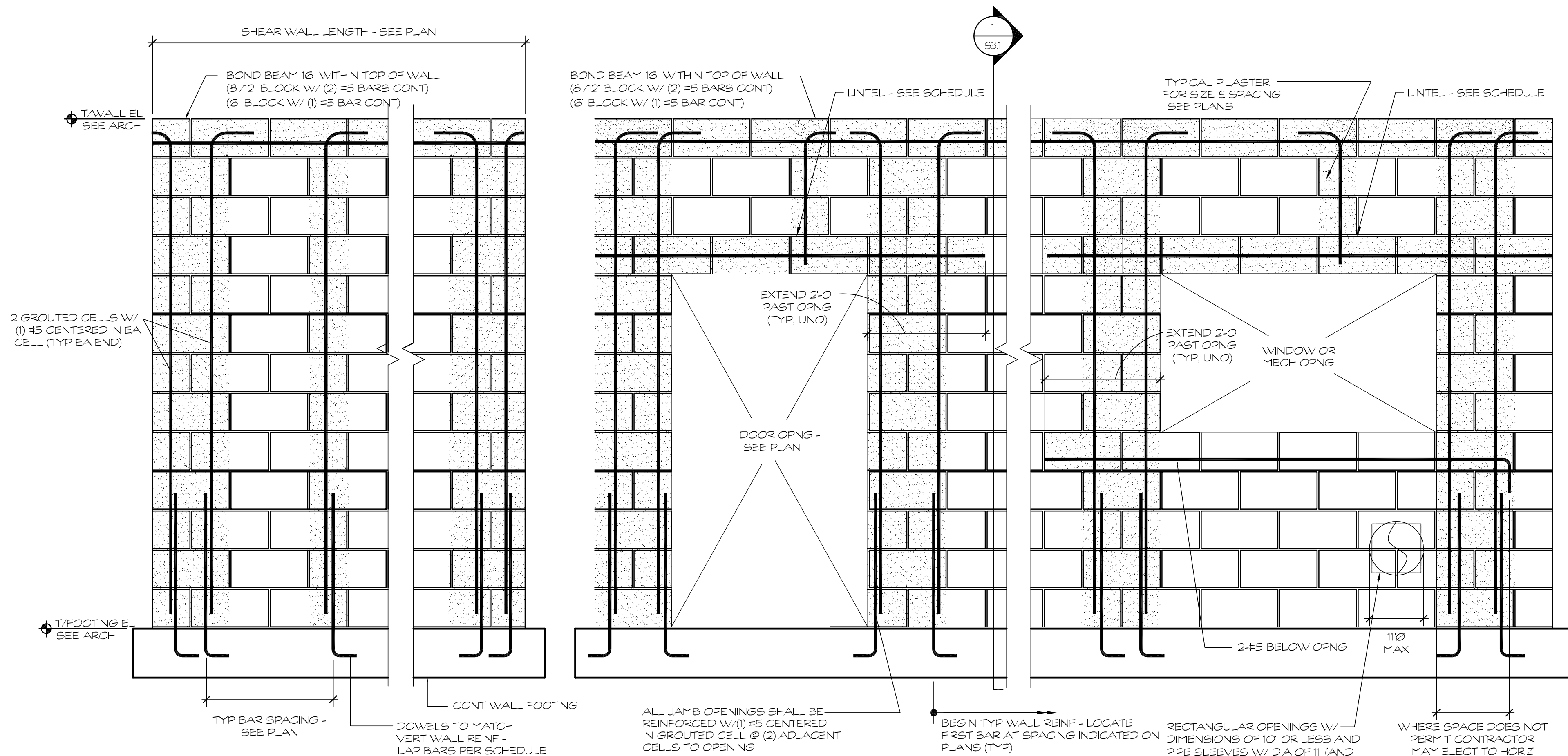
- NOTES:
1. SEE GENERAL NOTES FOR SPACING GUIDELINES FOR CONTROL JOINTS IN INTERIOR/EXTERIOR CMU WALLS.
  2. SEE ARCH FOR EXACT LOCATIONS OF CONTROL JOINTS
  3. DISCONTINUE HORIZONTAL REINFORCING AT CONTROL JOINT LOCATIONS

2 TYP CMU CONTROL JOINTS  
SCALE: NTS



- NOTES:
1. CORNER REINF. SHALL BE LAPPED WITH THE TYPICAL TRUSS TYPE HORIZ. REINF. AND EXTEND A MINIMUM OF 30" IN EACH DIRECTION AT THE INTERSECTION.
  2. SEE PLAN FOR SPACING OF TYPICAL HORIZ. REINF.

3 TYP CMU WALL CORNER INTERSECTION @ SHEARWALL  
SCALE: NTS



6 TYP CMU SHEARWALL ELEVATION  
SCALE: NTS

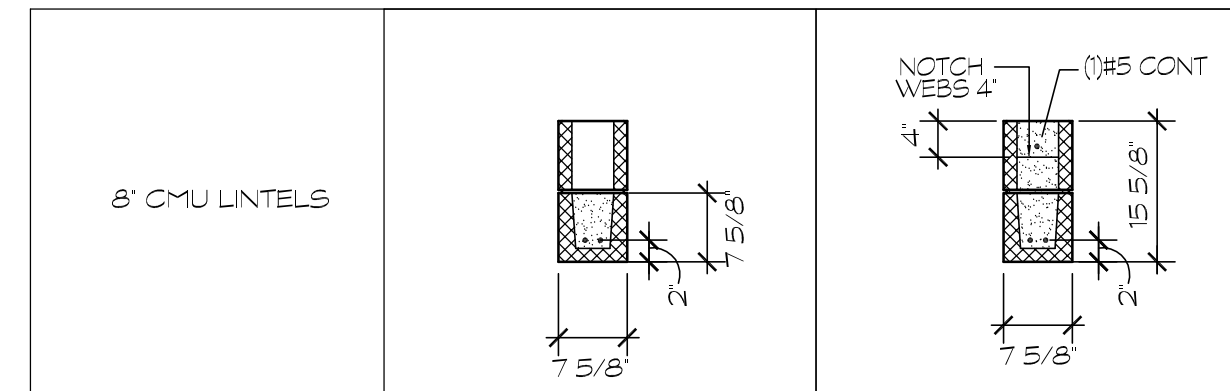
7 TYP CMU SHEARWALL ELEVATION WITH OPENINGS  
SCALE: NTS

OPENING WIDTH		STEEL LINTELS		MASONRY LINTELS			
		* STEEL FOR EACH 4' WALL THICKNESS	LINTEL DEPTH AND REINFORCING *	4" WALL	6" WALL	8" WALL	12" WALL
MIN.	MAX.		DEPTH				
-	2'-0"	L6 x 3 1/2 x 5/16 (SLV)	7 5/8"	(1) #4	(1)#4 BOTT.	(2)#5 BOTT.	(2)#5 BOTT.
2'-1"	3'-6"	L6 x 3 1/2 x 5/16 (SLV)	7 5/8"	(1) #4	(1)#4 BOTT.	(2)#5 BOTT.	(2)#5 BOTT.
3'-7"	5'-0"	L6 x 4 x 3/8 (SLV)	7 5/8"	(1) #4	(1)#5 BOTT.	(2)#5 BOTT.	(2)#5 BOTT.
5'-1"	6'-6"	L6 x 6 x 3/8	15 5/8"	-	(1)#5 BOTT.	(2)#5 BOTT.	(2)#6 BOTT.
6'-7"	8'-0"	L6 x 6 x 3/8	15 5/8"	-	(1)#5 BOTT.	(2)#5 BOTT.	(2)#6 BOTT.
8'-1"	10'-0"	L6 x 6 x 1/2	15 5/8"	-	(2)#5 BOTT.	(2)#5 BOTT.	(2)#6 BOTT.
10'-1"	12'-0"	L8 x 6 x 1/2 (LLV)	15 5/8"	-	(2)#5 BOTT.	(2)#5 BOTT.	(2)#6 BOTT.

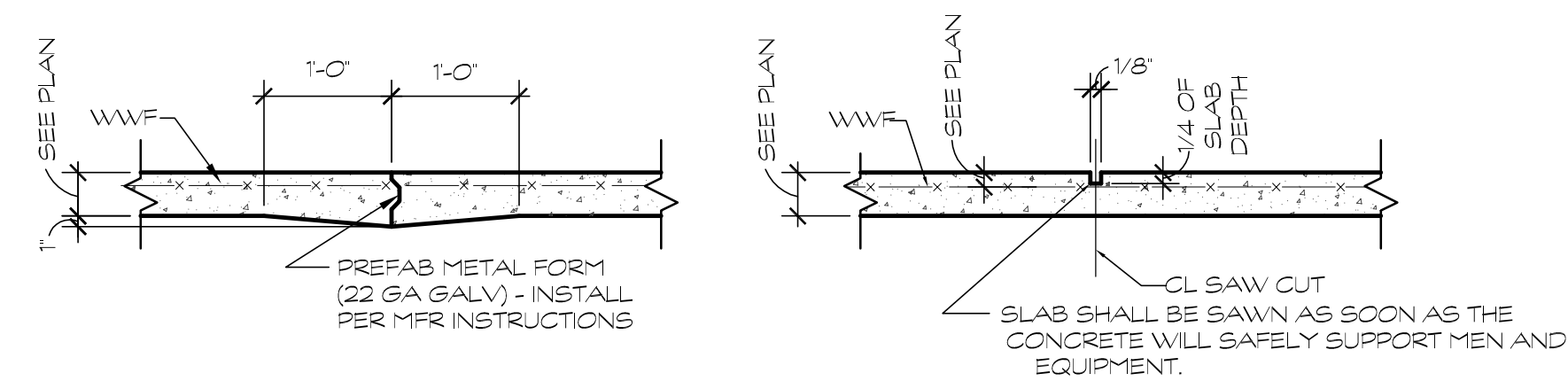
\* 8" BEARING EACH END FOR STEEL    \* 6" BEARING EACH END FOR U-BLOCK

- NOTES:
1. USE EITHER STEEL LINTEL OR MASONRY LINTEL (SEE ARCH HEAD DETAILS).
  2. THIS SCHEDULE TO BE USED UNLESS NOTED OTHERWISE.
  3. DO NOT USE THIS SCHEDULE IF CONCENTRATED LOAD IS APPLIED TO LINTEL.
  4. DO NOT USE THIS SCHEDULE IF HEIGHT OF MASONRY ABOVE OPENING IS LESS THAN HALF OF THE OPENING WIDTH.

4 MASONRY WALL LINTEL SCHEDULE  
SCALE: NTS



5 MASONRY LINTEL REINFORCING CONFIGURATIONS  
SCALE: NTS



TYP CONSTRUCTION JOINTS    TYP SAWED CONTROL JOINT

NOTE: USE CONSTRUCTION JOINT IN LIEU OF CONTROL JOINT WHENEVER A POUR STOP IS REQUIRED OR WHERE INDICATED ON THE PLAN.

8 TYP SLAB-ON-GRADE CONTROL JOINTS  
SCALE: 3/4"=1'-0"

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SHEET INDEX

TYPICAL  
SECTIONS &  
DETAILS

DRAWING NO.:

S3.1



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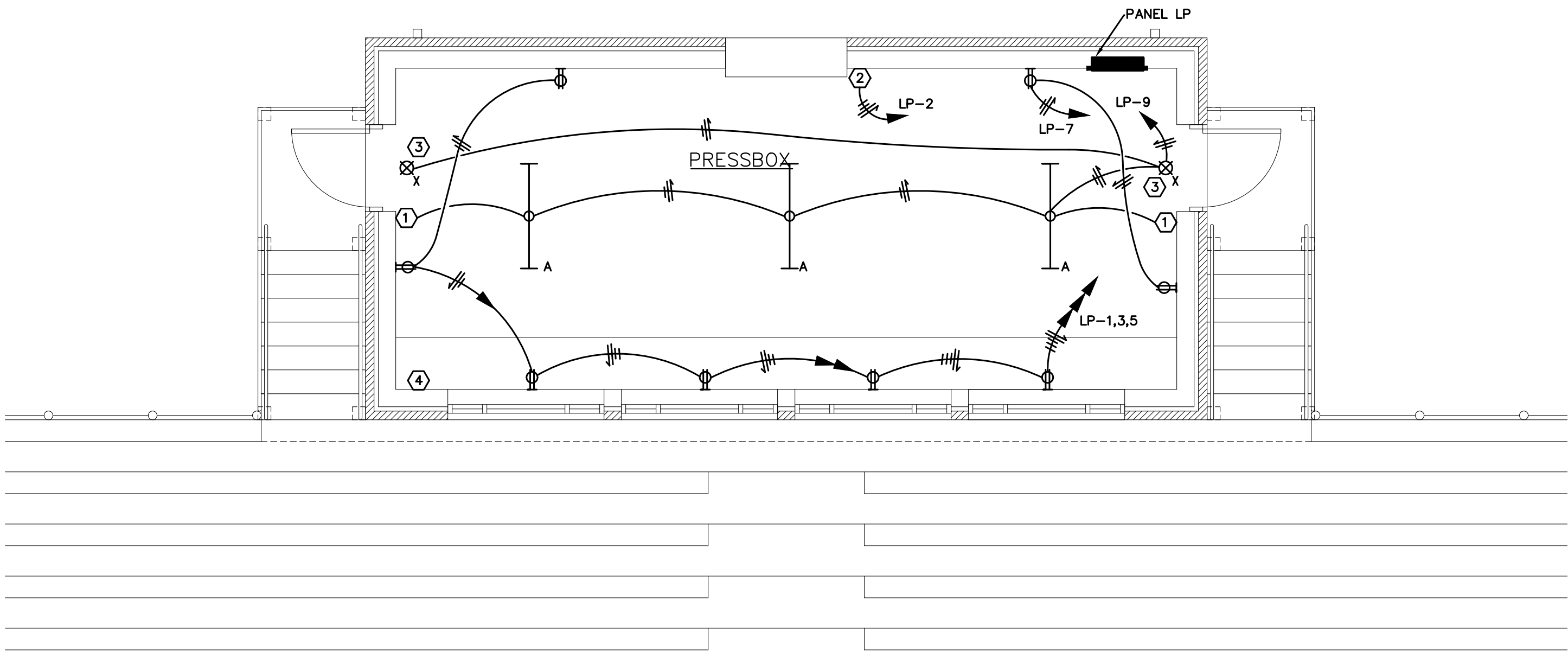
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PRESSBOX ELECTRICAL PLANS

DRAWING NO.:
E1.1

LIGHTING FIXTURE SCHEDULE:	
A	SURFACE MOUNTED FOUR FOOT LED, 120 VOLT, 90 CRI, 4000K COLOR TEMP, 0-10V DIMMING, 25 INPUT WATTS, 2500 LUMEN OUTPUT, CHAIN HANG TO 9' ABOVE THE FINISHED FLOOR. COLUMBIA LCL SERIES.
X	LED COMBO EXIT SIGN EMERGENCY LIGHT, RED LETTERS WITH WHITE FACE, 90 MINUTE EMERGENCY BATTERY PACK WITH TWO EMERGENCY LIGHTING HEADS, WALL MOUNT ABOVE THE DOOR, 120V. HUBBLE CCR SERIES.

SYMBOL SCHEDULE	
	LIGHTING FIXTURE - SURFACE MOUNTED LED
	EXIT LIGHT - WALL MOUNTED, ARROWS AS INDICATED
	15A, 125V, 2 POLE, 3 WIRE, GROUNDING, NEMA 5-15R DUPLEX RECEPT.
	TOGGLE SWITCH, SINGLE POLE
	120/208 OR 120/240 PANELBOARD AS SCHEDULED
	CONDUIT, HOMERUN (CROSS MARKS = # OF CONDUCTORS WHERE MORE THEN TWO)
	CONDUIT CONCEALED OVERHEAD IN FURRED CEILING, IN SLAB, OR IN WALLS
	(1) HOT (1) NEUTRAL (1) EQUIPMENT GROUND

PNL.BRD.		LP NO. OF CKT. SPACES:				20		MAIN DEVICE:					
PP VOLT.		208		AIC RATING:		22,000		TYPE: MCB					
NO. PH.		3		CABINET:		FLUSH		SIZE: 60A					
NO.	TA	WS	COND	DESCRIPT	TL	LOAD NO.	TA	WS	COND	DESCRIPT	TL	LOAD	
1	20	12	"1/2"	RECPT	R	1200	2	20/2	12	"1/2"	PTAC	C	3100
3	20	12	"1/2"	RECPT	R	1200	4	-----	-----	-----	-----		
5	20	12	"1/2"	RECPT	R	1200	6	20			SPARE		
7	20	12	"1/2"	RECPT	R	1200	8	20			SPARE		
9	20	12	"1/2"	LIGHTING	L	300	10						
11							12						
13							14						
15							16						
17							18						
19							20						
DESCRIPT.		DF	LOAD						DEMAND				
OTL PNLS.		1	0		VOLTAGE: 208						0		
RECEPT.		1	4800		PHASES: 3						4800		
LIGHT		1.25	300		SPARE % : 0.2						375		
HEAT		1	0								0		
COOL		1	3100								3100		
MOTOR		1	0								0		
OTHER		1	0								0		
SPARE		1	1640								1640		
		CONN. LOAD		AMP				DEM. LOAD		AMP			
		9840		27				9915		28			
NOTES:													



① PRESSBOX PLAN  
SCALE: 1/4" = 1'-0"

- KEYNOTES:
- 0-10V LED DIMMER THREE WAY SWITCH. FURNISH AND INSTALL THE SWITCH TO CONTROL THE LIGHTS IN THIS SPACE. RUN LOW VOLTAGE CONTROL CABLE IN CONDUIT TO DIMMABLE BALLASTS.
  - FURNISH AND INSTALL A 208 VOLT NEMA 6-20R RECEPTACLE FOR THE HVAC UNIT. COORDINATE THE EXACT LOCATION WITH THE HVAC CONTRACTOR PRIOR TO ROUGH-IN.
  - WALL MOUNT THE EXIT SIGN ABOVE THE DOOR. COORDINATE THE EXACT MOUNTING HEIGHT WITH THE ARCHITECT PRIOR TO ROUGH-IN.
  - FURNISH AND INSTALL A 3" CONDUIT FROM THE PRESS BOX TO BELOW THE EXISTING VISITOR SIDE STANDS FOR FUTURE OWNER CABLEING.
  - FURNISH AND INSTALL A NEW 60A/3P BREAKER IN THE EXISTING SERVICE 400A MAIN 208/120 VOLT THREE PHASE FOUR WIRE PANEL. NEW BREAKER SHALL BE U.L. LISTED FOR USE IN THE EXISTING PANEL AND MATCH ALL EXISTING RATINGS. EXISTING PANEL LOCATED BELOW THE VISITOR STANDS. COORDINATE THE EXACT LOCATION WITH THE OWNER/ARCHITECT PRIOR TO BID.
  - CONTRACTOR TO VERIFY THE CONDITION AND SIZE OF THE EXISTING SERVICE GROUND. IF SERVICE GROUND IS NOT INSTALLED OR SIZE OF GROUND IS LESS THE A #1/00 THEN RUN #1/0 GROUND CONDUCTOR TO THREE 5/8" X 10' COPPER/CLAD GROUND RODS SPACED TEN FEET APART ON THE EXTERIOR OF THE BUILDING, TO THE BUILDING STEEL.
  - MAINTAIN THE EXISTING SERVICE FEEDER TO PANEL.

